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Welded Connections
A Primer for Engineers



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Course Description

21.7 Special Welding Applications December 3, 2019

This session covers atypical welding applications for buildings. The live webinar presents practical, straightforward solutions to challenging situations including: welding of steel headed stud anchors, welding on coated steels, welding AESS, welding on existing structures, welding heavy sections, welding under high restraint, field welding, heat shrinking, cold temperature applications and more.



Learning Objectives

- Describe tolerance issues that may affect welded connections in AESS structures.
- Identify welded HSS connections where member configurations could prevent proper welding, inspection or assembly.
- List considerations for welded connections on rolled heavy shapes.
- List consideration for welded connections in cold temperature applications



Night School 21 Course Schedule

- | | |
|------------------|--|
| 10/8/2019 | 1. Introduction and Weld Processes |
| 10/15/2019 | 2. Principles of Welded Connections |
| 10/29/2019 | 3. Welded Connection Details |
| 11/5/2019 | 4. Metallurgy and Cracking |
| 11/19/2019 | 5. Fatigue of Welded Connections |
| 11/26/2019 | 6. Seismic Welding Issues |
| 12/3/2019 | 7. Special Welding Applications |
| 12/10/2019 | 8. Problems and Fixes |



Night School 21 Welded Connections -- A Primer for Engineers

Session 7: Special Welding Applications
December 3, 2019



Duane K. Miller, PE, ScD
Manager of Engineering Services and Welding
Design Consultant



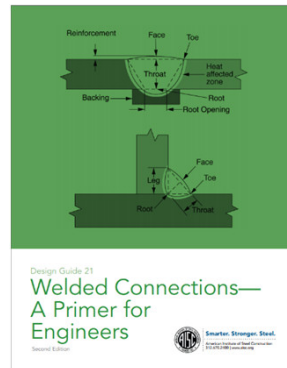
SPECIAL WELDING APPLICATIONS



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AISC Design Guide 21, 2nd Edition

Welded Connections—
A Primer for Engineers



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SPECIAL WELDING APPLICATIONS



Chapter 14 Special Welding Applications



- 14.1 Welding of Steel Headed Stud Anchors
- 14.2 Welding on Galvanized Steels
- 14.3 Welding on Primed and Painted Steel
- 14.4 Welding on Heavy Shapes
- 14.5 Welding on Highly Restrained Members
- 14.6 Welding HSS



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SPECIAL WELDING APPLICATIONS



- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.10 Welds and Mechanical Fasteners
- 14.11 Welding on Members to be Hot-Dip Galvanized
- 14.12 Cold Temperature Applications
- 14.13 Deck Welding



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SPECIAL WELDING APPLICATIONS

- 14.14 Welding on In-Place Embed Plates
- 14.15 Heat Shrinking
- 14.16 Buttering



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SPECIAL WELDING APPLICATIONS

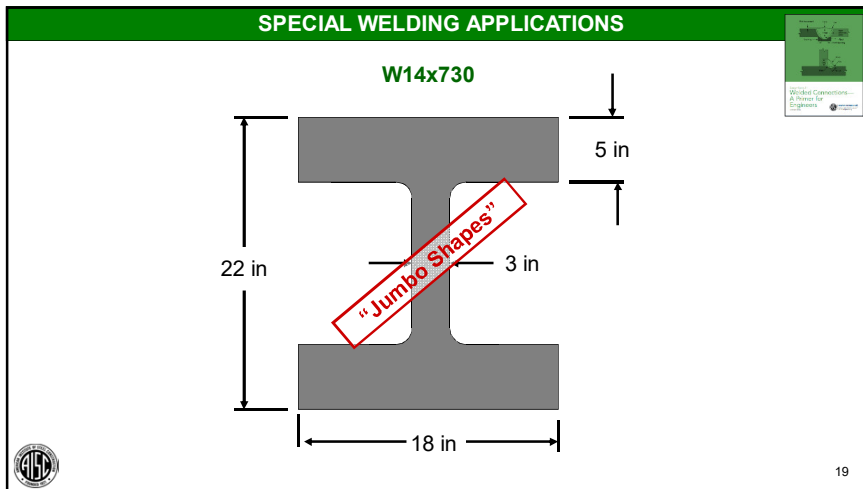
Today's Webinar

- ➔ 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering

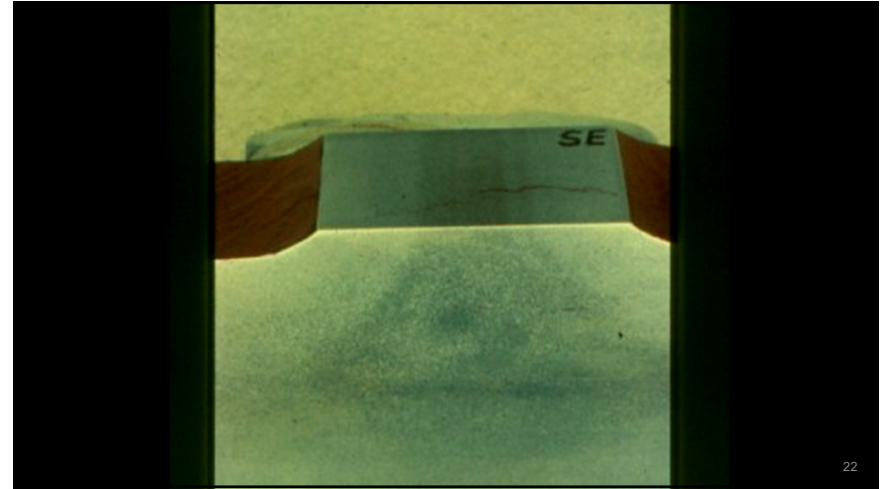
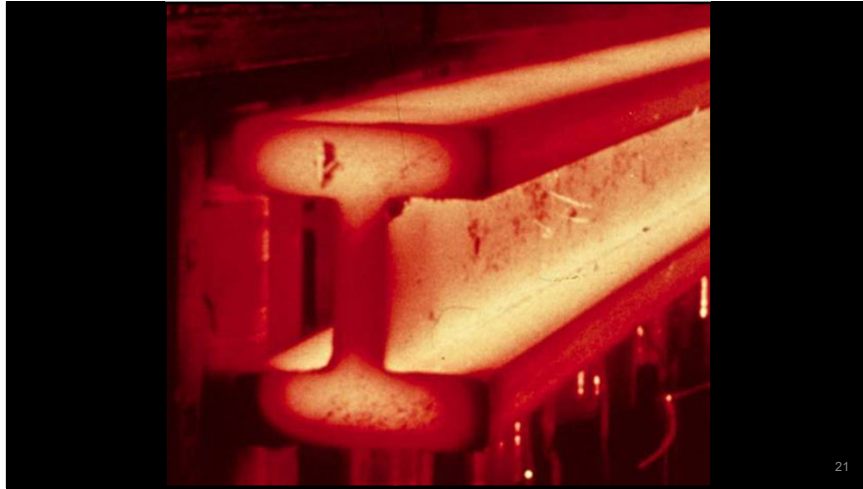


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A slide titled "AISC 360-16 SPECIFICATION" with a blue header. The main content is "Commentary A3" and "1c. Rolled Heavy Shapes". The text states: "The web-to-flange intersection and the web center of heavy hot-rolled shapes, as well as the interior portions of heavy plates, may contain a more coarse grain structure and/or lower notch toughness material than other areas of these products." Below this text are three red-bordered boxes: "Segregation", "Reduced Rolling", and "Slower Cooling". The AISC logo is in the bottom left, and the number '20' is in the bottom right.



AISC 360-16 Specification for Structural Steel Buildings

Commentary A3

1c. Rolled Heavy Shapes

This characteristic is not detrimental to suitability for compression members or for non-welded members. However, when heavy cross sections are joined by splices or connections using complete-joint-penetration groove welds that extend through the coarser and/or low notch-tough interior portions, tensile strains induced by welding may result in cracking.

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Commentary A3

1a. ASTM Designations

Rolled Steel is anisotropic, especially insofar as ductility is concerned; therefore, weld contraction strain in the region of highly restrained welded connections may exceed the strength of the material if special attention is not given to material selection, details, workmanship and inspection.

Material Details Workmanship Inspection

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AISC 360-16 Specification for Structural Steel Buildings

Material

**CHAPTER A
GENERAL PROVISIONS**

A3. MATERIAL

1c. Rolled Heavy Shapes

ASTM A6/A6M hot-rolled shapes with a flange thickness exceeding 2 in. (50 mm) are considered to be rolled heavy shapes. Rolled heavy shapes used as members subject to primary (computed) tensile forces due to tension or flexure and spliced or connected using complete-joint-penetration groove welds that fuse through the thickness of the flange or the flange and the web, shall be specified as follows. The structural design documents shall require that such shapes be supplied with Charpy V-notch (CVN) impact test results in

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AISC 360-16 Specification for Structural Steel Buildings

Material

**CHAPTER A
GENERAL PROVISIONS**

A3. MATERIAL

1c. Rolled Heavy Shapes (cont'd)

The structural design documents shall require that such shapes be supplied with Charpy V-notch (CVN) impact test results in accordance with ASTM A6/A6M, Supplementary Requirement S30, Charpy V-Notch Impact Test for Structural Shapes—Alternate Core Location. The impact test shall meet a minimum average value of 20 ft-lb (27 J) absorbed energy at a maximum temperature of +70°F (+21°C).

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ASTM A6 Supplementary Requirements

Material

*S30 Charpy V-Notch Impact Tests for Structural Shapes:
Alternate Core Location*

20 ft-lbs (27J) @ +70 °F (+21 °C)

$t_f/4$

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AISC 360-16 Specification for Structural Steel Buildings

Material

CHAPTER A

GENERAL PROVISIONS



A3 MATERIAL

1d. Built-Up Heavy Shapes

Built-up cross sections consisting of plates with a thickness exceeding 2 in. (50 mm) are considered built-up heavy shapes. Built-up heavy shapes used as members subject to primary (computed) tensile forces due to tension or flexure and spliced or connected to other members using complete-joint-penetration groove welds that fuse through the thickness of the plates, shall be specified as follows. The structural design documents shall require that the steel be supplied with Charpy V-notch impact test results in accordance with ASTM A6/A6M,


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Material

CHAPTER A

GENERAL PROVISIONS



A3 MATERIAL

1d. Built-Up Heavy Shapes (cont'd)

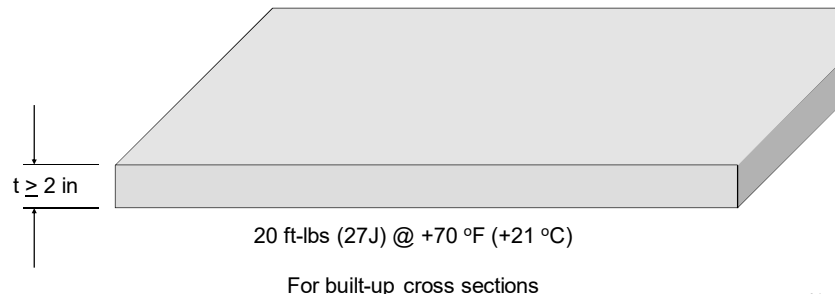
The structural design documents shall require that the steel be supplied with Charpy V-notch impact test results in accordance with ASTM A6/A6M, Supplementary Requirement S5, Charpy V-Notch Impact Test. The impact test shall be conducted in accordance with ASTM A673/A673M, Frequency P, and shall meet a minimum average value of 20 ft-lb (27 J) absorbed energy at a maximum temperature of +70°F (+21°C).

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ASTM A6 Supplementary Requirements

Material

S5 Charpy V-Notch Impact Test



20 ft-lbs (27J) @ +70 °F (+21 °C)

For built-up cross sections


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Material

CHAPTER J

DESIGN OF CONNECTIONS



J2. 6. Filler Metal Requirements

Filler metal with a specified minimum Charpy V-notch toughness of 20 ft-lb (27 J) at 40°F (4°C) or lower shall be used in the following joints:

- (a) CJP groove welded T- and corner joints with steel backing left in place, subject to tension normal to the effective area, unless the joints are designed using the nominal strength and resistance factor or safety factor, as applicable, for a PJP groove weld
- (b) CJP groove welded splices subject to tension normal to the effective area in heavy sections, as defined in Sections A3.1c and A3.1d


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Material

J2.6 Filler Metal Requirements

20 ft-lbs (27J) @ +40 °F (+4 °C)




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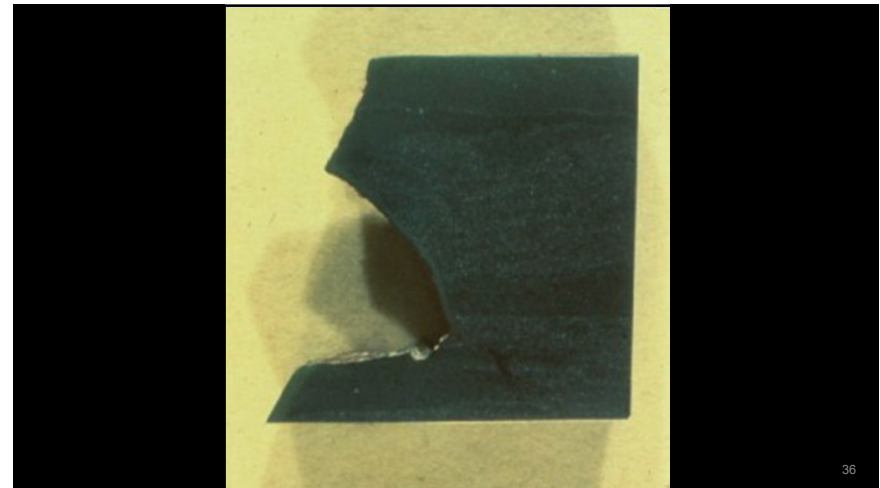
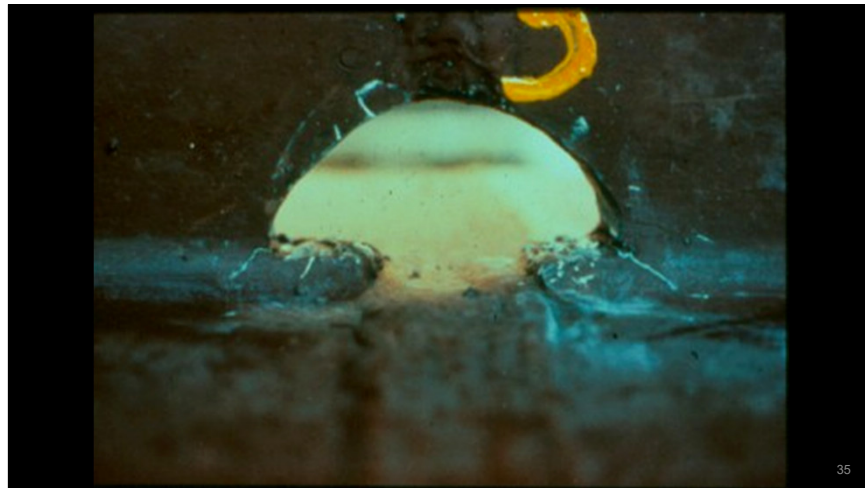
Commentary A3

1c. Rolled Heavy Shapes

To minimize the potential for fracture, the notch toughness requirements of Section A3.1c must be used in conjunction with good design and fabrication procedures. Specific requirements are given in Sections J1.5, J1.6, J2.6 and J2.7.



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Details

CHAPTER J

DESIGN OF CONNECTIONS

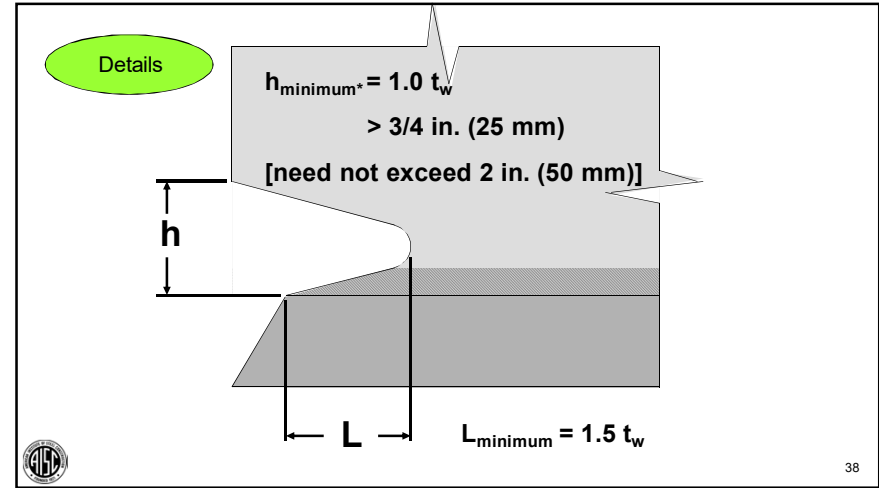


J1.6. Weld Access Holes

Weld access holes shall meet the following requirements:

- (a) All weld access holes required to facilitate welding operations shall be detailed to provide room for weld backing as needed.
- (b) The access hole shall have a length from the toe of the weld preparation not less than 1-1/2 times the thickness of the material in which the hole is made, nor less than 1-1/2 in. (38 mm).
- (c) The access hole shall have a height not less than the thickness of the material with the access hole, nor less than 3/4 in. (19 mm), nor does it need to...

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AISC 360-16 Specification for Structural Steel Buildings

Workmanship

CHAPTER M

FABRICATION AND ERECTION



M2.2. Thermal Cutting

For beam copes and weld access holes in which the curved part of the access hole is thermally cut in ASTM A6/A6M hot-rolled shapes with a flange thickness exceeding 2 in. (50 mm) and welded built-up shapes with material thickness greater than 2 in. (50 mm), a preheat temperature of not less than 150°F (66°C) shall be applied prior to thermal cutting. The thermally cut surface of access holes in ASTM A6/A6M hot-rolled shapes with a flange thickness exceeding 2 in. (50 mm) and built-up shapes with a material thickness greater than 2 in. (50 mm) shall be ground.

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SPECIAL WELDING APPLICATIONS

Today's Webinar

- 14.4 Welding on Heavy Shapes
- ➔ 14.6 Welding HSS
- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering





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SPECIAL WELDING APPLICATIONS

14.6 Welding HSS

- ➔ • Connections and HSS member size
- Overall configuration
- Cutting and preparing HSS


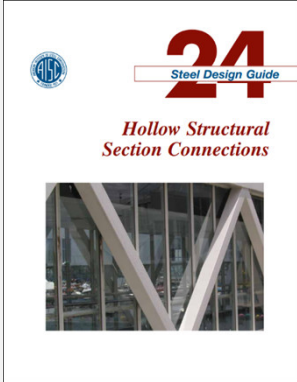


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Design Guide 24

Hollow Structural Section Connections

by
Packer, Sherman, Lecce





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Design Guide 24 Hollow Structural Section Connections

**Chapter 1
Introduction**

“Connections usually have been the challenging aspect for the designer of structures that involve HSS...Note that, in many cases, the local strength of the HSS at the connection is an integral part—and perhaps a limitation--of the design.”

HSS member size may need to be increased to accommodate connection design.





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Design Guide 24 Hollow Structural Section Connections

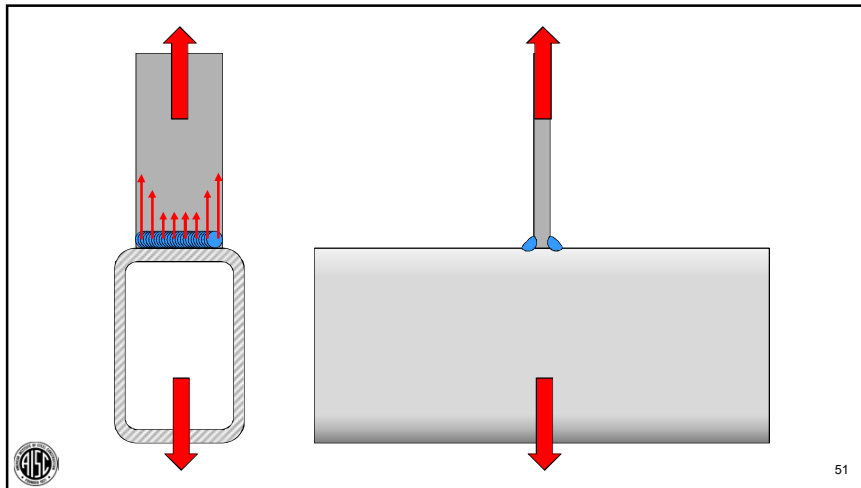
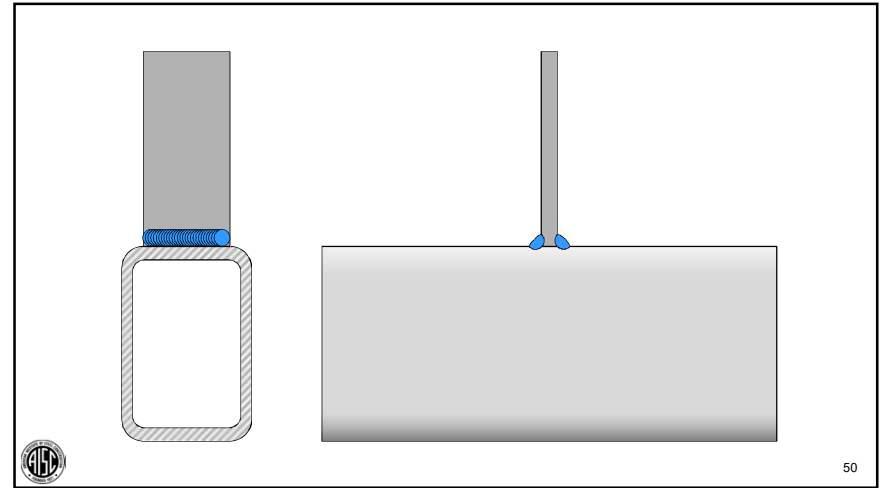
Chapter 2
Welding

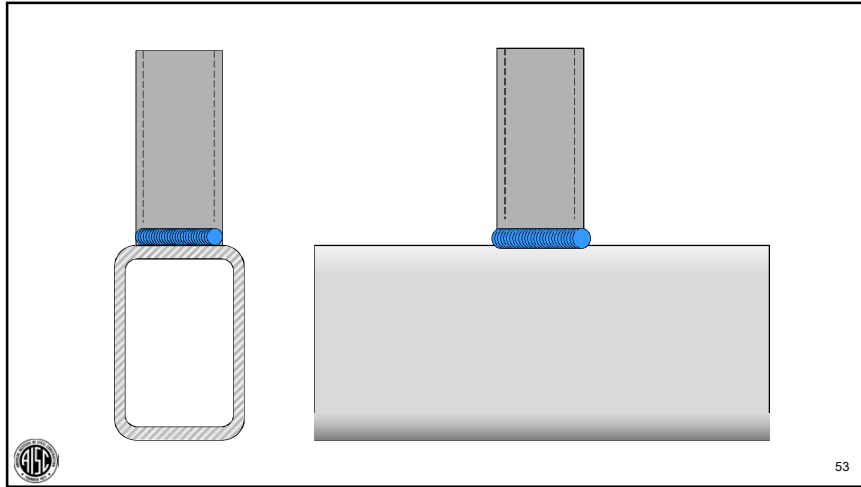
2.4 EFFECTIVE WELD LENGTH

Due to the variation in flexural stiffness of the wall across the width of a rectangular HSS, a force transmitted through a weld is often not uniformly distributed. This can be accounted for in design using an effective weld length approach. This reduced effective length applies both to the weld and to the force in the connected element.

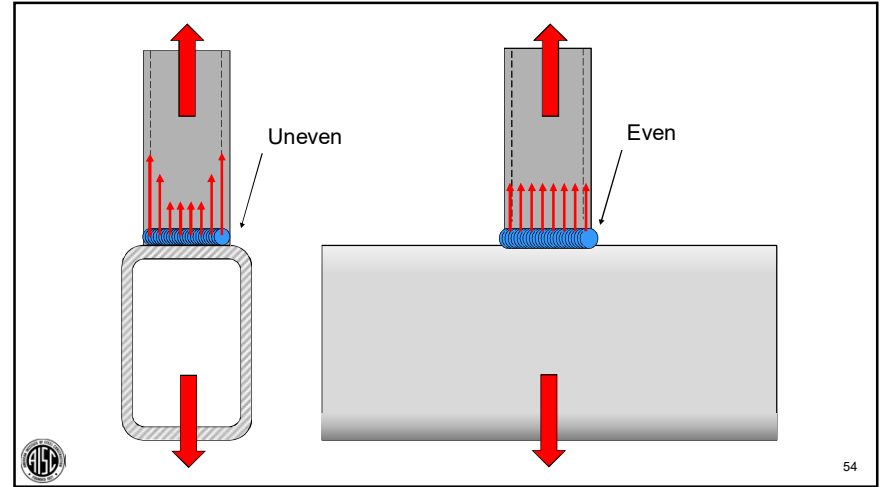


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AISC 360:16 Specification for Structural Steel Buildings

TABLE K2.1
Available Strength of Plate-to-Round HSS Connections

Connection Type	Connection Available Strength	Plate Bending
Transverse Plate T- and Cross-Connections	Limit State: HSS Local Yielding	
	Plate Axial Load	In-Plane Out-of-Plane
	$R_n \sin \theta = F_y t^2 \left(\frac{6.5}{1 - 0.81 \frac{D}{t^2}} \right) Q_t$ (K2-1a)	$M_n = 0.56 R_n$ (K2-1b)
Longitudinal Plate T-, Y- and Cross-Connections	Limit State: HSS Plastification	
	Plate Axial Load	In-Plane Out-of-Plane
	$R_n \sin \theta = 5.5 F_y t^2 \left(1 + 0.25 \frac{l_b}{D} \right) Q_t$ (K2-2a)	$M_n = 0.8 l_b R_n$ (K2-2b)
Functions		
$Q_t = 1$ for HSS (connecting surface) in tension $= 1.0 - 0.3U$ (1 = U) for HSS (connecting surface) in compression (K2-3)		
$U = \frac{P_c}{F_y A_c} + \frac{M_c}{F_y S_x}$ (K2-4)		
where P_c and M_c are determined on the side of the joint that has the lower compression stress. P_c and M_c refer to required strengths in the HSS: $P_c = P_c$ for LRFD, and P_c for ASD; $M_c = M_c$ for LRFD, and M_c for ASD.		

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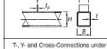
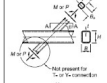
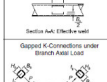
Longitudinal Plate T-, Y- and Cross-Connections

Limit State: HSS Plastification		
Plate Axial Load	In-Plane	Out-of-Plane
$R_n \sin \theta = 5.5 F_y t^2 \left(1 + 0.25 \frac{l_b}{D} \right) Q_t$ (K2-2a)	$M_n = 0.8 l_b R_n$ (K2-2b)	-
$\phi = 0.90$ (LRFD) $\Omega = 1.67$ (ASD)		

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AISC 360:16 Specification for Structural Steel Buildings

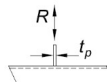
TABLE K5.1
Effective Weld Properties for
Connection to Rectangular HSS

Connection Type	Weld Properties
 Transverse Plate T- and Cross-Connections under Plate Axial Load	Effective Weld Properties $l_e = 2L_e$ (K5-4) where l_e = total effective weld length for welds on both sides of the transverse plate
 T, Y- and Cross-Connections under Branch Axial Load or Bending	Effective Weld Properties $l_e = 2t_p$ (K5-5) $R_p = \frac{1}{2} \left[\frac{M_p}{A_p} + \frac{1}{2} \left(\frac{M_p}{A_p} \right) \right] + L_e \left(\frac{M_p}{A_p} \right)$ (K5-6) $R_p = \frac{1}{2} \left[\frac{M_p}{A_p} + \frac{1}{2} \left(\frac{M_p}{A_p} \right) \right] + L_e \left(\frac{M_p}{A_p} \right)$ (K5-7) Not present for t_p to determine
 Gapped K-Connections under Branch Axial Load	Effective Weld Properties When $\alpha < 50^\circ$: $l_e = \frac{2t_p}{\sin \alpha} - 1.2t_p$ and $2(B_p - 1.2t_p)$ (K5-8) When $\alpha > 60^\circ$: $l_e = \frac{2t_p}{\sin \alpha} - 1.2t_p$ and $B_p - 1.2t_p$ (K5-9) When $50^\circ < \alpha < 60^\circ$, linear interpolation shall be used to determine l_e . (t _p = plate thickness)

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AISC 360:16 Specification for Structural Steel Buildings

TABLE K5.1
Effective Weld Properties for
Connections to Rectangular HSS

Connection Type	Weld Properties
 Transverse Plate T- and Cross-Connections under Plate Axial Load	<div style="border: 1px solid red; padding: 5px; display: inline-block;">Effective Weld Properties</div> $l_e = 2B_e$ (K5-4) where l_e = total effective weld length for welds on both sides of the transverse plate

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SPECIAL WELDING APPLICATIONS

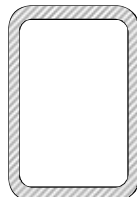
14.6 Welding HSS

- Connections and HSS member size
- ➔ • Overall configuration
- Cutting and preparing HSS

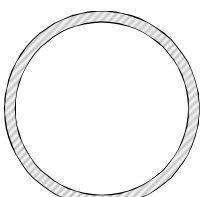
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SPECIAL WELDING APPLICATIONS

Rectangular

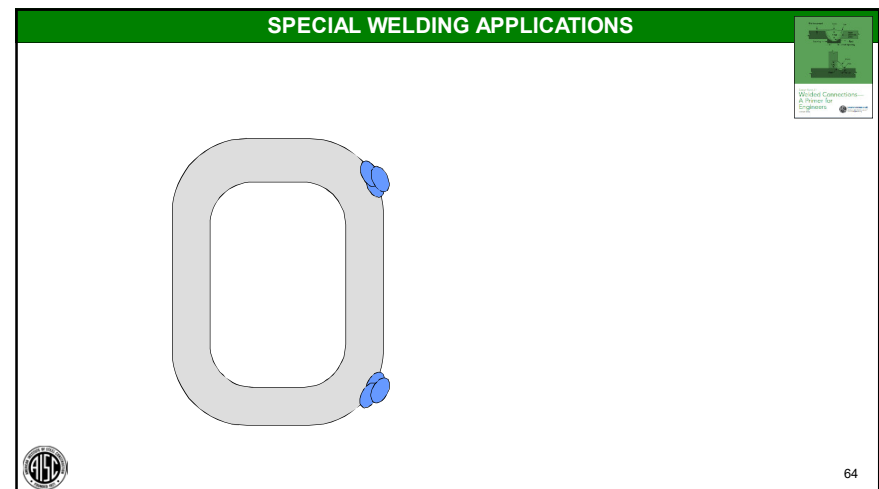
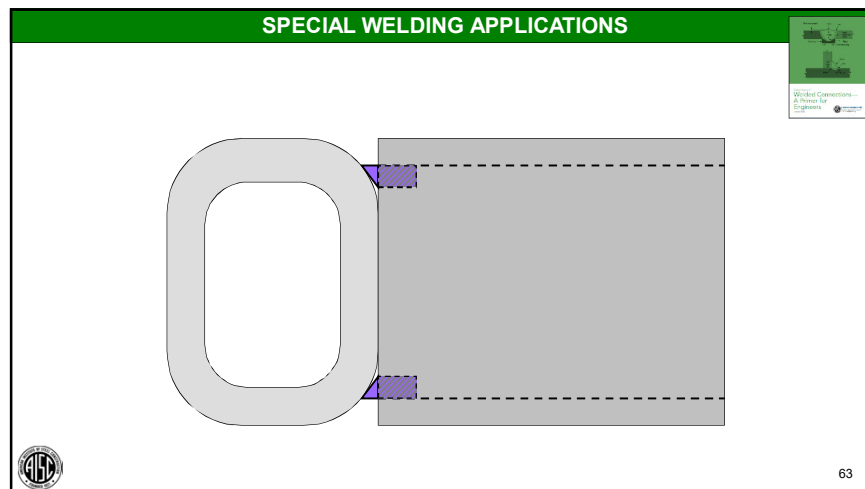
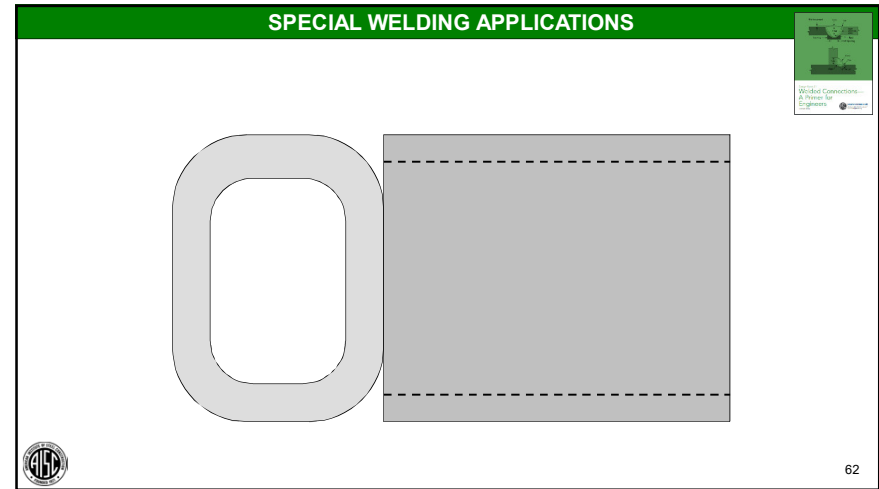
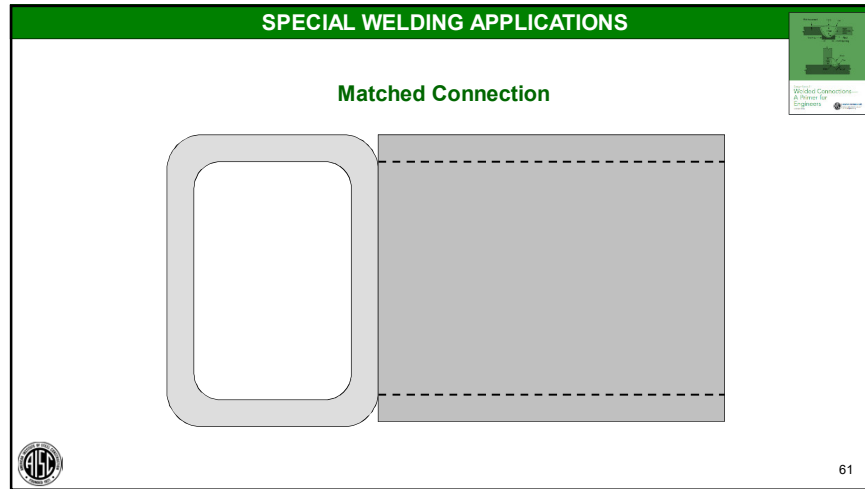


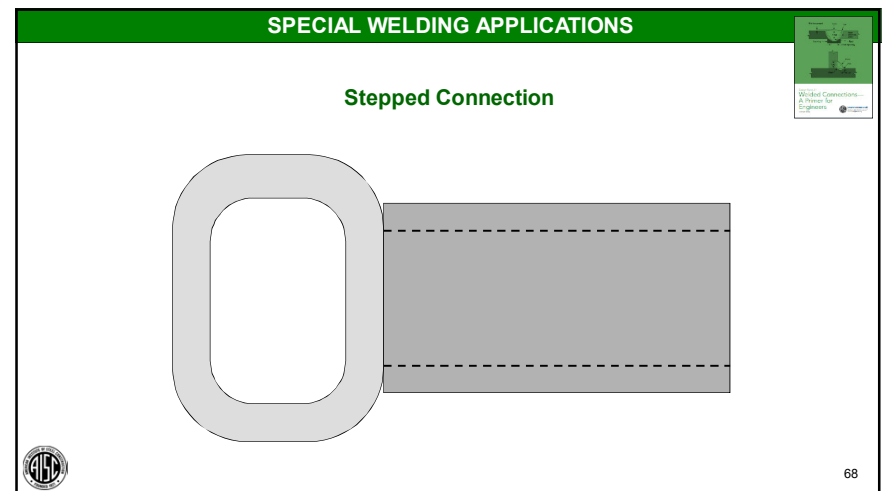
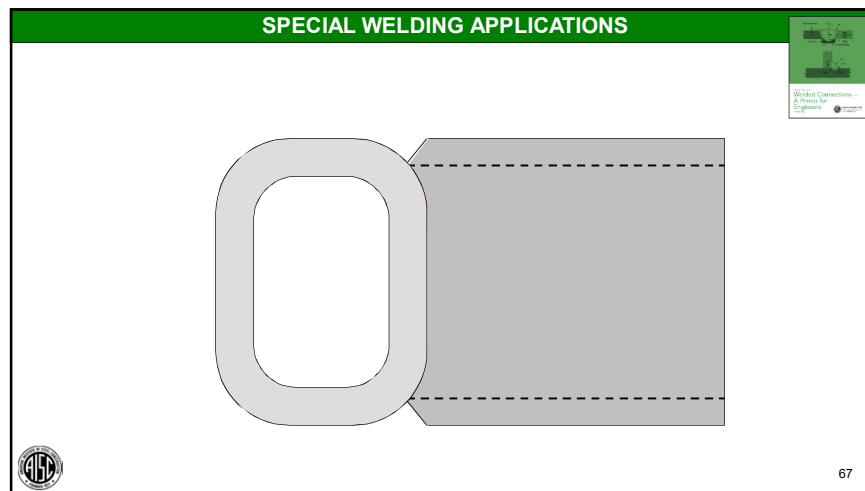
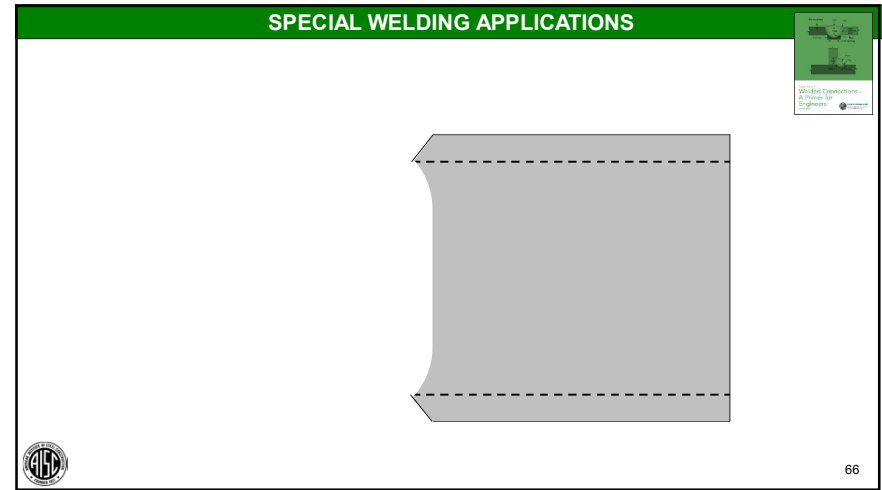
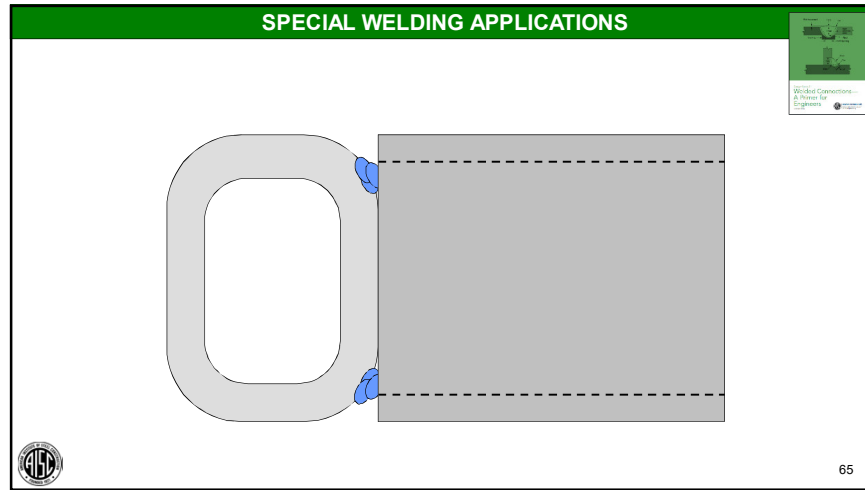
Round



Rectangular is nearly always easier to fabricate

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SPECIAL WELDING APPLICATIONS

14.6 Welding HSS


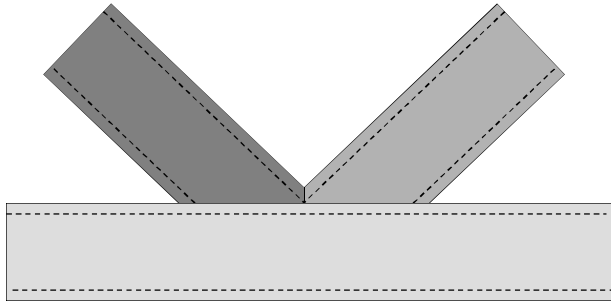
- Connections and HSS member size
- ➔ • Overall configuration
- Cutting and preparing HSS



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SPECIAL WELDING APPLICATIONS


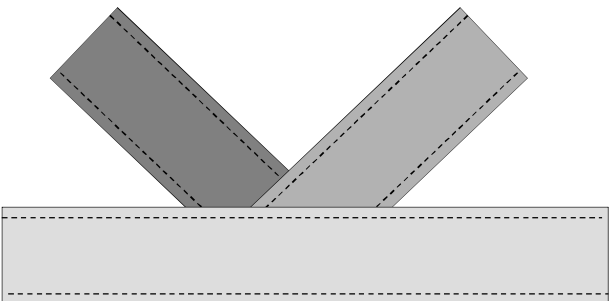
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SPECIAL WELDING APPLICATIONS

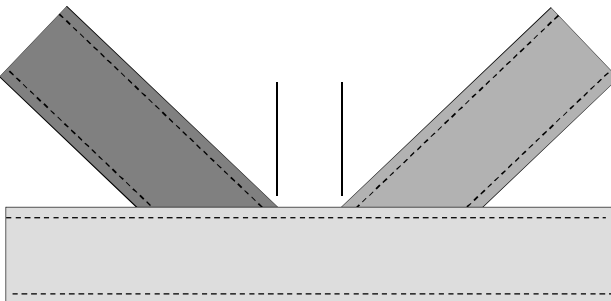
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SPECIAL WELDING APPLICATIONS

Gapped



Preferred

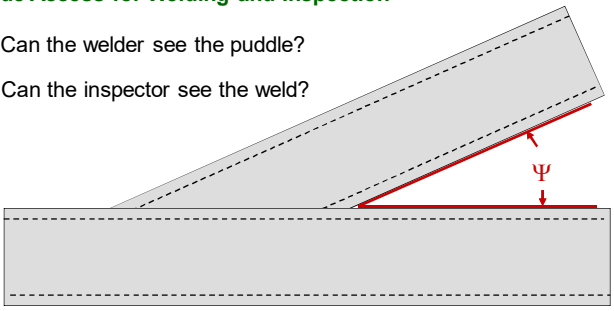


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SPECIAL WELDING APPLICATIONS

Provide Access for Welding and Inspection

- Can the welder see the puddle?
- Can the inspector see the weld?



$\Psi = 30^\circ$ minimum

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The diagram shows a cross-section of a horizontal HSS member on the left and a sloped HSS member on the right. A red line indicates the angle Ψ between the horizontal member's top surface and the sloped member's top surface. A red arrow points to this angle. The sloped member is shown with a dashed line representing its original position before being rotated.

SPECIAL WELDING APPLICATIONS

14.6 Welding HSS

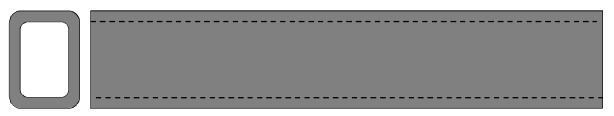
- Connections and HSS member size
- Overall configuration
- Cutting and preparing HSS

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The diagram shows a cross-section of a horizontal HSS member on the left and a sloped HSS member on the right. A red line indicates the angle Ψ between the horizontal member's top surface and the sloped member's top surface. A red arrow points to this angle. The sloped member is shown with a dashed line representing its original position before being rotated.

SPECIAL WELDING APPLICATIONS

Box HSS

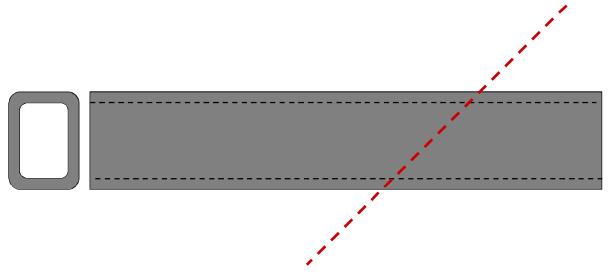


75

The diagram shows a side view of a box HSS member. On the left end, there is a rectangular cutout. The member is shown with dashed lines representing its original shape before the cutout was made.

SPECIAL WELDING APPLICATIONS

Box HSS




76


The diagram shows a side view of a box HSS member. A diagonal dashed red line indicates a cutout through the member. The member is shown with dashed lines representing its original shape before the cutout was made.

SPECIAL WELDING APPLICATIONS

Box HSS



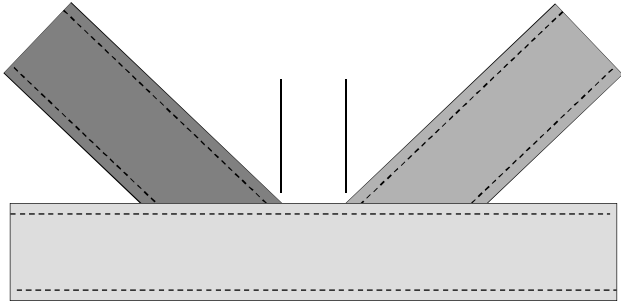
Welded Connections...
A Primer for
Engineers




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SPECIAL WELDING APPLICATIONS

Gapped




Welded Connections...
A Primer for
Engineers




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SPECIAL WELDING APPLICATIONS

Box HSS




Welded Connections...
A Primer for
Engineers




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SPECIAL WELDING APPLICATIONS

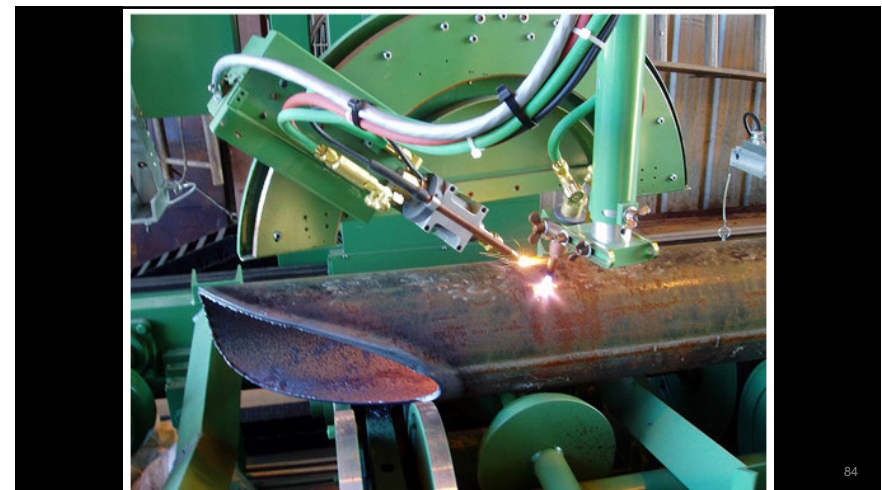
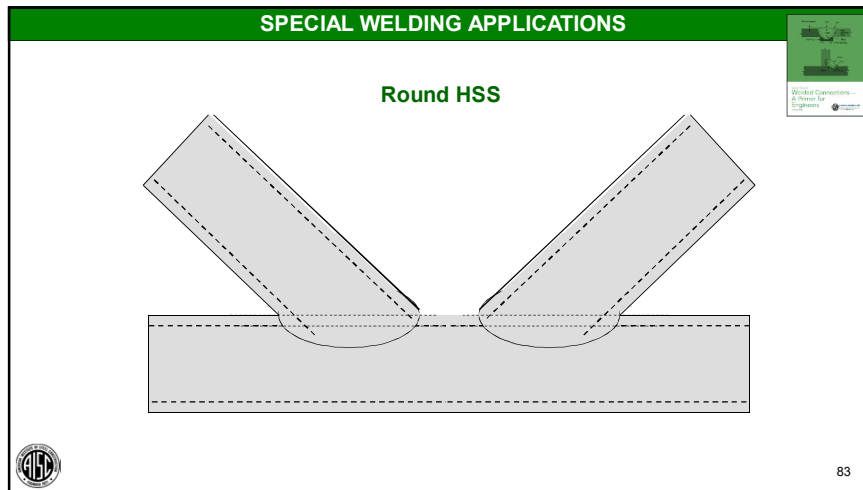
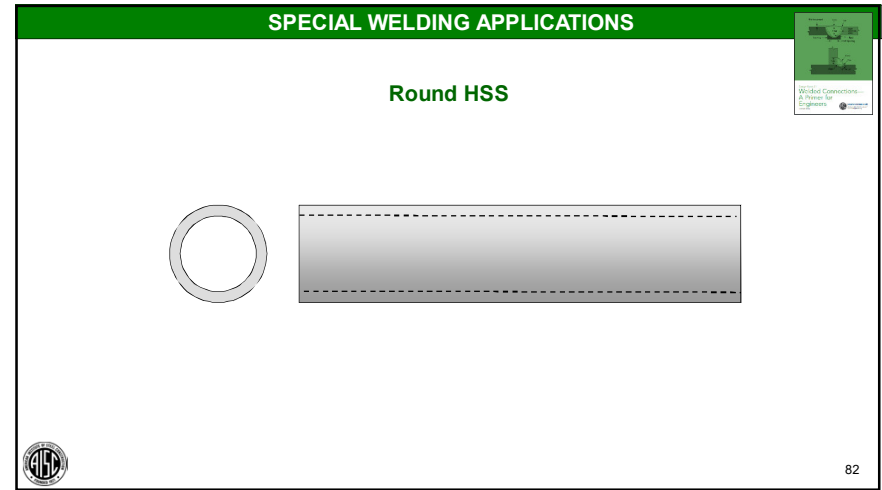
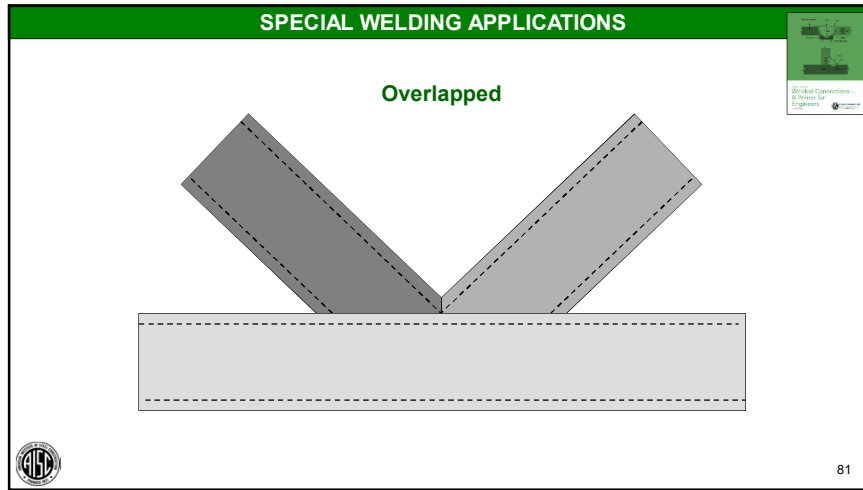
Box HSS



Welded Connections...
A Primer for
Engineers

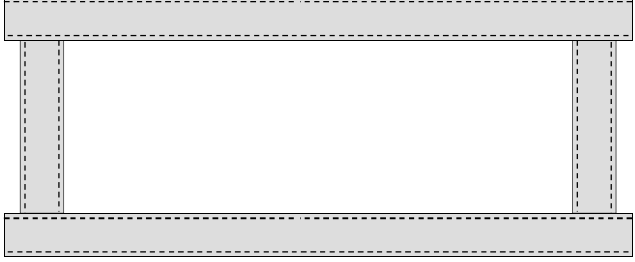


80




SPECIAL WELDING APPLICATIONS

Box HSS



Welded Connections...
A Primer for
Engineers

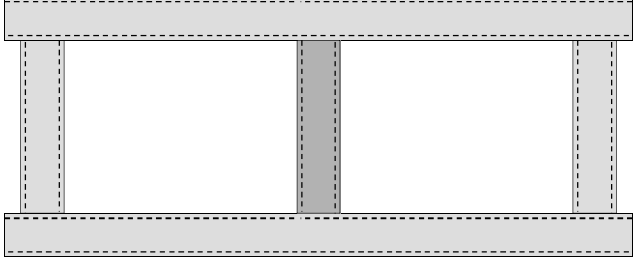


85


This diagram shows a cross-section of a Box Hollow Structural Section (HSS) with two vertical stiffeners. The stiffeners are positioned symmetrically on either side of the vertical centerline, extending from the top flange to the bottom flange. Dashed lines indicate the original boundaries of the HSS flanges and the stiffeners.

SPECIAL WELDING APPLICATIONS

Box HSS



Welded Connections...
A Primer for
Engineers

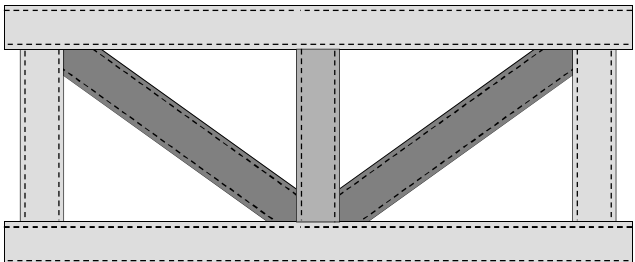


86


This diagram shows a cross-section of a Box HSS with three vertical stiffeners. One stiffener is located at the vertical centerline, and two others are positioned symmetrically on either side. All stiffeners extend from the top flange to the bottom flange. Dashed lines indicate the original boundaries of the HSS flanges and the stiffeners.

SPECIAL WELDING APPLICATIONS

Box HSS



Welded Connections...
A Primer for
Engineers

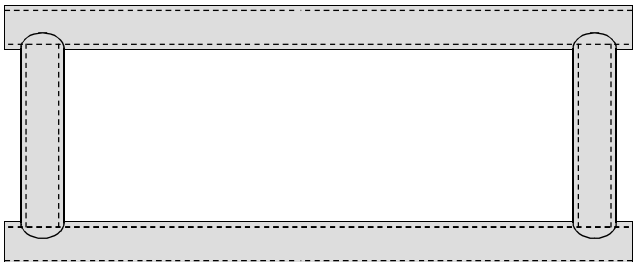


87


This diagram shows a cross-section of a Box HSS with a diagonal stiffener. The stiffener is a single member that runs from the top flange on one side to the bottom flange on the opposite side, forming a V-shape with the vertical centerline. Dashed lines indicate the original boundaries of the HSS flanges and the diagonal stiffener.

SPECIAL WELDING APPLICATIONS

Round HSS

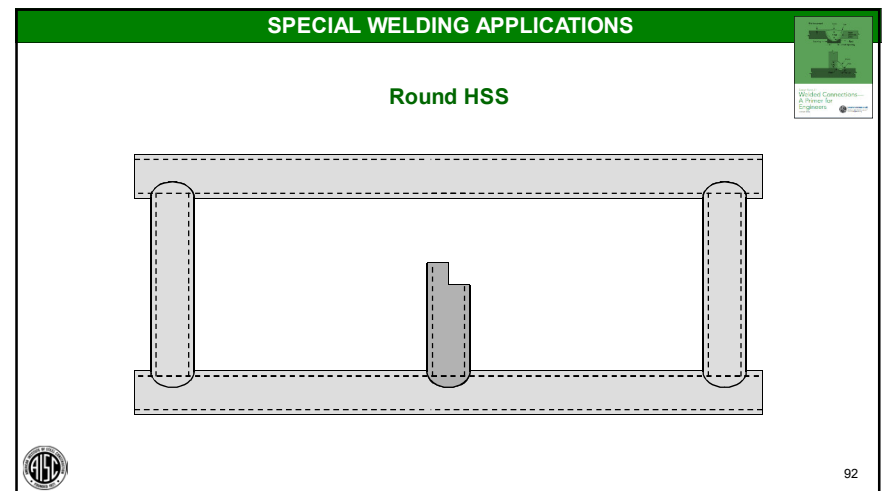
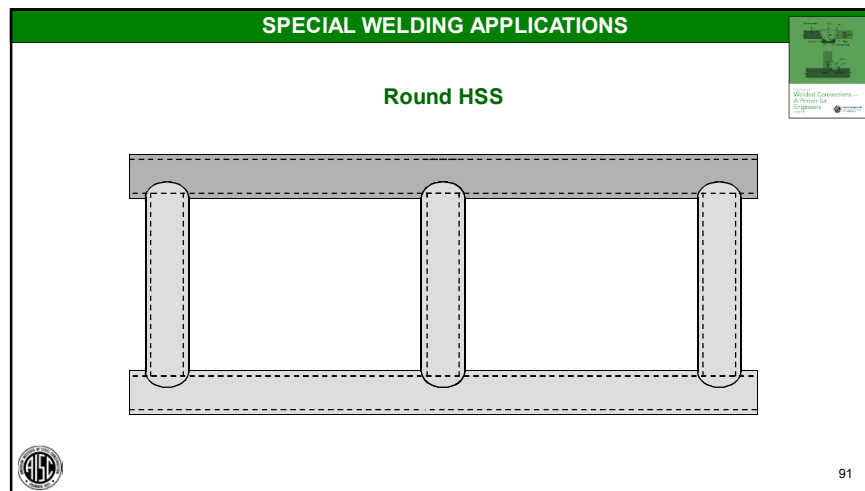
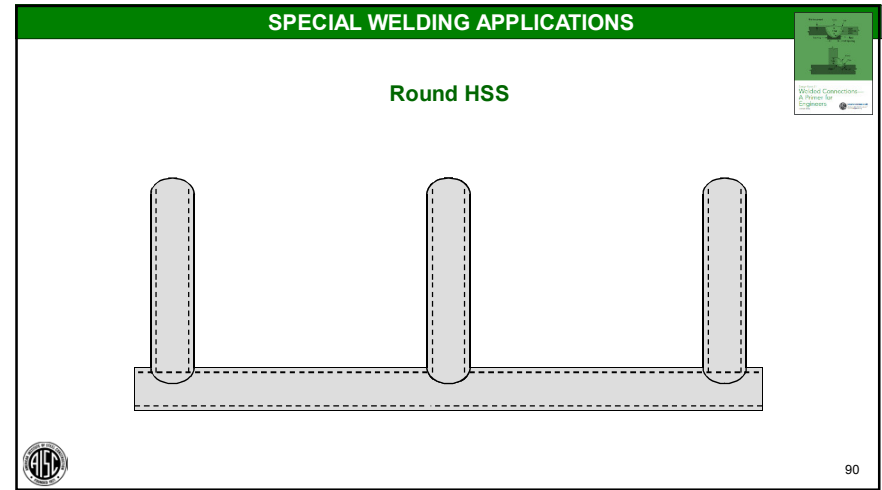
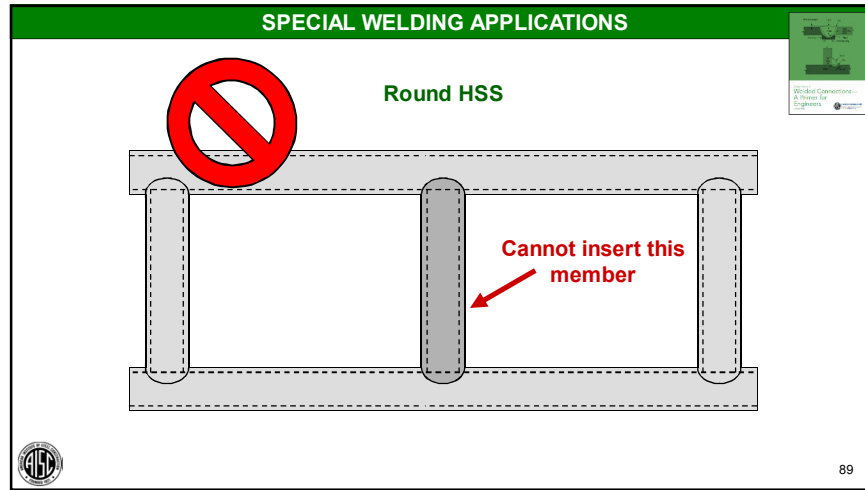


Welded Connections...
A Primer for
Engineers



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This diagram shows a cross-section of a Round Hollow Structural Section (HSS) with two vertical stiffeners. The stiffeners are positioned symmetrically on either side of the vertical centerline, extending from the top flange to the bottom flange. Dashed lines indicate the original boundaries of the HSS flanges and the stiffeners.



SPECIAL WELDING APPLICATIONS

Round HSS

Welded Connections...
A Primer for
Engineers

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SPECIAL WELDING APPLICATIONS

Round HSS

Welded Connections...
A Primer for
Engineers

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AWS D1.1:2015 STRUCTURAL WELDING CODE

9. Tubular Structures

9.1 General
This Clause supplements Clauses 1–8. The specific requirements of Clause 9 apply only to tubular connections. For design purposes this clause shall be used with the applicable requirements of Clause 2, Part A. All provisions of Clause 9 apply to static applications and cyclic applications, with the exception of the fatigue provisions of 9.2.7, which are unique to cyclic applications.
This clause is divided into parts, as follows:
Part A — Design of Tubular Connections
Part B — Prequalification of Welding Procedure Specifications (WPSs)
Part C — Welding Procedure Specification (WPS) Qualification
Part D — Performance Qualification
Part E — Fabrication

9.2.3 Tubular Section Limitations. Limitations on diameter/thickness for circular sections, and largest flat width/thickness ratio for box sections, beyond which local buckling or other local failure modes shall be considered, shall be in conformance with the governing design code. Limits of applicability for the criteria given in 9.6 shall be observed as follows:
(1) Circular tubes: $D/t < 3300/F_y$ [for F_y in ksi], $22\ 800/F_y$ (for F_y in MPa)
(2) Box section gap connections: $D/t \leq 210/\sqrt{F_y}$ [for F_y in ksi], $550/\sqrt{F_y}$ (for F_y in MPa) but not more than 35
(3) Box section overlap connections: $D/t \leq 190/\sqrt{F_y}$ [for F_y in ksi], $500/\sqrt{F_y}$ (for F_y in MPa)

9.2.4 Welds Stresses. The allowable stresses in welds shall not exceed those given in Table 9.2, or as allowed

Welded Connections...
A Primer for
Engineers

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SPECIAL WELDING APPLICATIONS

Welding HSS: Summary

- Welds are usually unevenly stressed
- Connection details may control member sizes
- Round vs square/rectangular: box tubes usually easier
- Cutting and assembly
- Access for welding and inspection
- May also have AESS concerns

Welded Connections...
A Primer for
Engineers

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SPECIAL WELDING APPLICATIONS

Today's Webinar

- 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- ➔ 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering



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SPECIAL WELDING APPLICATIONS

Welding AESS

- ➔ Defining “pretty”
- Use of Mock-ups
- Technical issues
- Note requirements on drawings



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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

AISC 303-16

Code of Standard Practice for Steel Buildings and Bridges

Section 10.
Architecturally Exposed Structural Steel

TABLE 10.1
AESS Category Matrix

Category	AESS C	AESS 4	AESS 3	AESS 2	AESS 1	SSS
Id	Custom Elements	Showcase Elements	Feature Elements in close view	Feature Elements not in close view	Basic Elements	Standard Structural Steel
1.1	*	*	*	*	*	*
1.2	*	*	*	*	*	*
1.3	*	*	*	*	*	*
1.4	*	*	*	*	*	*
1.5	*	*	*	*	*	*
2.1	*	*	*	optional	*	*
2.2	*	*	*	*	*	*
2.3	*	*	*	*	*	*
2.4	*	*	*	*	*	*
3.1	*	*	*	*	*	*
3.2	*	*	*	*	*	*
3.3	*	*	*	*	*	*
3.4	*	*	*	*	*	*
3.5	*	*	*	*	*	*
3.6	*	optional	optional	*	*	*
4.1	*	*	*	*	*	*
4.2	*	*	*	*	*	*
4.3	*	*	*	*	*	*
4.4	*	*	*	*	*	*

Use Note:
1.1 Prior to blast cleaning, grease and oil are removed by solvent cleaning to meet SSPC-SP 1.
1.2 Rough surfaces are debrued and ground smooth. Sharp edges resulting from flame cutting, grinding and especially shearing are softened.
1.3 Interferometric testing is made continuously during with additional testing required on back filler. For permanent quality.

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

TABLE 10.1
AESS Category Matrix

Category	AESS C	AESS 4	AESS 3	AESS 2	AESS 1	SSS
Id	Custom Elements	Showcase Elements	Feature Elements in close view	Feature Elements not in close view	Basic Elements	Standard Structural Steel
1.1	*	*	*	*	*	*
1.2	*	*	*	*	*	*
1.3	*	*	*	*	*	*
1.4	*	*	*	*	*	*
1.5	*	*	*	*	*	*
2.1	*	*	*	optional	*	*
2.2	*	*	*	*	*	*
2.3	*	*	*	*	*	*
2.4	*	*	*	*	*	*
3.1	*	*	*	*	*	*
3.2	*	*	*	*	*	*
3.3	*	*	*	*	*	*
3.4	*	*	*	*	*	*
3.5	*	*	*	*	*	*
3.6	*	optional	optional	*	*	*
4.1	*	*	*	*	*	*
4.2	*	*	*	*	*	*
4.3	*	*	*	*	*	*
4.4	*	*	*	*	*	*

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

TABLE 10.1
AESS Category Matrix

Category	AESS C	AESS 4	AESS 3	AESS 2	AESS 1	SSS
Id	Custom Elements	Showcase Elements	Feature Elements in close view	Feature Elements not in close view	Basic Elements	Standard Structural Steel
1.1	*	*	*	*	*	*
1.2	*	*	*	*	*	*
1.3	*	*	*	*	*	*
1.4	*	*	*	*	*	*
1.5	*	*	*	*	*	*
2.1	*	*	*	optional	*	*
2.2	*	*	*	*	*	*
2.3	*	*	*	*	*	*
2.4	*	*	*	*	*	*

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

TABLE 10.1
AESS Category Matrix

Category	AESS C	AESS 4	AESS 3	AESS 2	AESS 1	SSS
Id	Custom Elements	Showcase Elements	Feature Elements in close view	Feature Elements not in close view	Basic Elements	Standard Structural Steel
1.1	*	*	*	*	*	*
1.2	*	*	*	*	*	*
1.3	*	*	*	*	*	*
1.4	*	*	*	*	*	*
1.5	*	*	*	*	*	*
2.1	*	*	*	optional	*	*
2.2	*	*	*	*	*	*
2.3	*	*	*	*	*	*
2.4	*	*	*	*	*	*

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

1.1	Surface preparation to SSPC-SP 6			•	•	•	•
1.2	Sharp edges ground smooth			•	•	•	•
➔ 1.3	Continuous weld appearance			•	•	•	•
1.4	Standard structural bolts			•	•	•	•
➔ 1.5	Weld spatters removed			•	•	•	•
➔ 2.1	Visual samples			•	•	optional	
➔ 2.2	One-half standard fabrication tolerances			•	•	•	
2.3	Fabrication marks not apparent			•	•	•	
➔ 2.4	Welds uniform and smooth			•	•	•	
3.1	Mill marks removed			•	•		
➔ 3.2	Butt and plug welds ground smooth and filled			•	•		
3.3	HSS weld seam oriented for reduced visibility			•	•		
3.4	Cross sectional abutting surface aligned			•	•		
➔ 3.5	Joint gap tolerances minimized			•	•		
➔ 3.6	All welded connections			optional	optional		
4.1	HSS seam not apparent			•			
➔ 4.2	Welds contoured and blended			•			
4.3	Surfaces filed and sanded			•			
➔ 4.4	Weld show-through minimized			•			

Many welding-related issues

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SPECIAL WELDING APPLICATIONS

Welding AESS

- Defining "pretty"
- ➔ • Use of Mock-ups
- Technical issues
- Note requirements on drawings

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

1.1	Surface preparation to SSPC-SP 6			•	•	•	•
1.2	Sharp edges ground smooth			•	•	•	•
1.3	Continuous weld appearance			•	•	•	•
1.4	Standard structural bolts			•	•	•	•
1.5	Weld spatters removed			•	•	•	•
➔ 2.1	Visual samples			•	•	optional	
2.2	One-half standard fabrication tolerances			•	•	•	
2.3	Fabrication marks not apparent			•	•	•	
2.4	Welds uniform and smooth			•	•	•	
3.1	Mill marks removed			•	•		
3.2	Butt and plug welds ground smooth and filled			•	•		
3.3	HSS weld seam oriented for reduced visibility			•	•		
3.4	Cross sectional abutting surface aligned			•	•		
3.5	Joint gap tolerances minimized			•	•		
3.6	All welded connections			optional	optional		
4.1	HSS seam not apparent			•			
4.2	Welds contoured and blended			•			
4.3	Surfaces filed and sanded			•			
4.4	Weld show-through minimized			•			

Mock-ups

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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

SECTION 10. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL

10.1.2. A mock-up shall be required for AESS 3, 4 and C. If a mock-up is to be used in other AESS categories, it shall be specified in the *contract documents*. When required, the nature and extent of the mock-up shall be specified in the *contract documents*. Alternatively, when a mock-up is not practical, the first piece of an element or *connection* can be used to determine acceptability.


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AISC 303-16 Code of Standard Practice for Steel Buildings and Bridges

SECTION 10. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL

Commentary:
Generally, a mock-up is produced and approved in the shop and subsequently placed in the field. **The acceptability of the mock-up can be affected by many factors, including distance of view, lighting and finishing.** The expectations for the **location and conditions of the mock-up at time of approval** should be defined in the *contract documents*.

Suggestion: View mock-up at the same distance as the viewer will be from the final welded connection in service.




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SPECIAL WELDING APPLICATIONS

Welding AESS


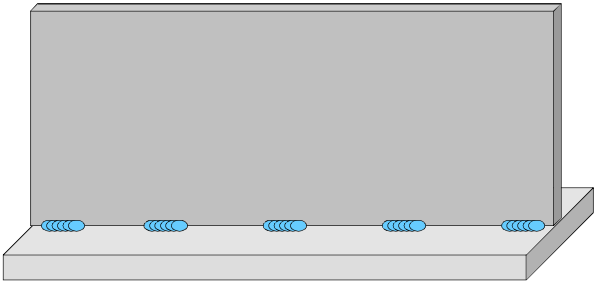
- Defining "pretty"
- Use of Mock-ups
- ➔ • Technical issues
- Note requirements on drawings



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SPECIAL WELDING APPLICATIONS

Sufficient for strength


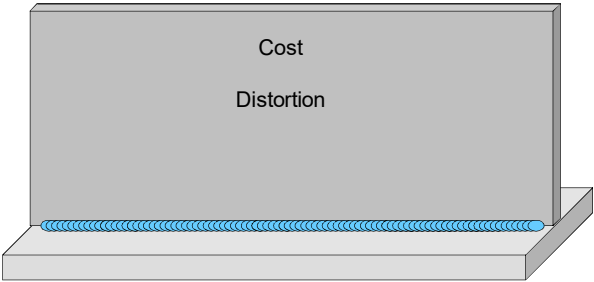


115

SPECIAL WELDING APPLICATIONS

May be required for AESS

Cost
Distortion

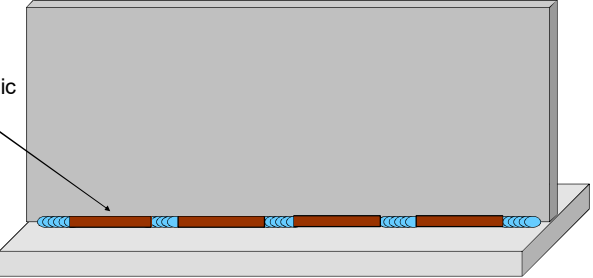


116

SPECIAL WELDING APPLICATIONS

Sufficient for strength

Non-metallic filler



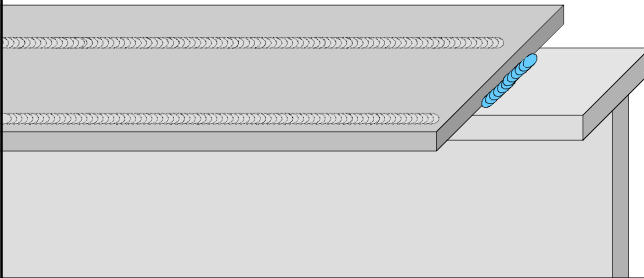
The diagram shows a cross-section of a butt joint between two steel plates. The joint is filled with a non-metallic filler material, indicated by a red and blue hatched pattern. The filler is applied in a way that provides sufficient strength for the connection.

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SPECIAL WELDING APPLICATIONS

D1.1 code required practice*

*modified in 2015 edition



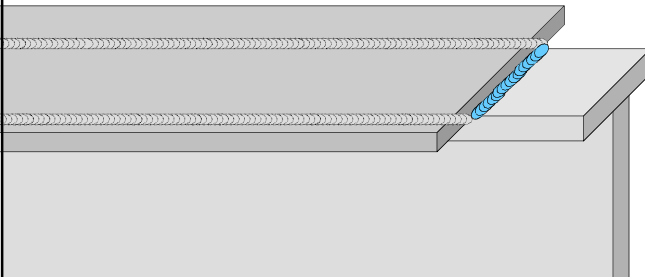
The diagram shows a lap joint between two steel plates. The joint is filled with a non-metallic filler material, indicated by a red and blue hatched pattern. The filler is applied in a way that meets the D1.1 code required practice, which was modified in the 2015 edition.

118

SPECIAL WELDING APPLICATIONS

May be AESS specified practice

Workmanship Concerns
Inspection Concerns

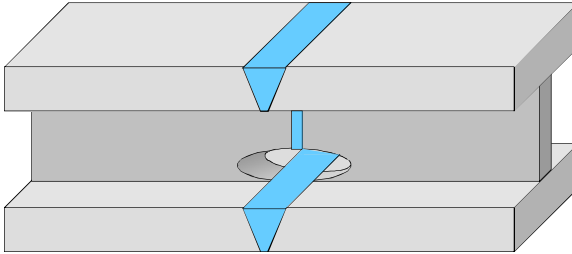


The diagram shows a lap joint between two steel plates. The joint is filled with a non-metallic filler material, indicated by a red and blue hatched pattern. The filler is applied in a way that may be specified by AESS, but it has workmanship and inspection concerns.

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SPECIAL WELDING APPLICATIONS

Code required practice

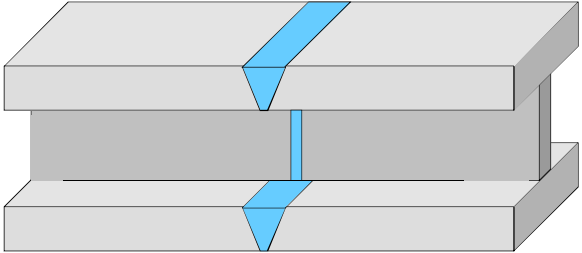


The diagram shows a lap joint between two steel plates. The joint is filled with a non-metallic filler material, indicated by a red and blue hatched pattern. The filler is applied in a way that meets the code required practice.

120

SPECIAL WELDING APPLICATIONS

May be AESS specified practice



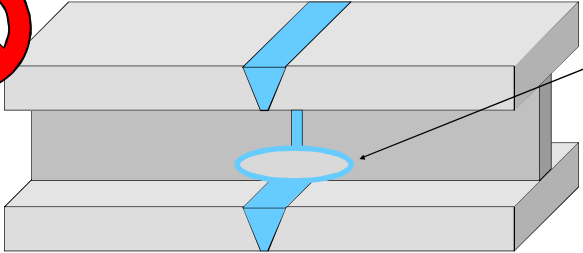
How can flange CJP groove weld be made?

121

This diagram shows a cross-section of a flange CJP groove weld. Two horizontal plates are joined by a vertical groove weld. The weld is shown in blue, with a vertical weld line in the center of the groove. The plates are shown in grey. The text 'May be AESS specified practice' is at the top, and 'How can flange CJP groove weld be made?' is at the bottom. The AISC logo is in the bottom left corner, and the number '121' is in the bottom right corner.

SPECIAL WELDING APPLICATIONS

May be AESS specified practice



Welded insert

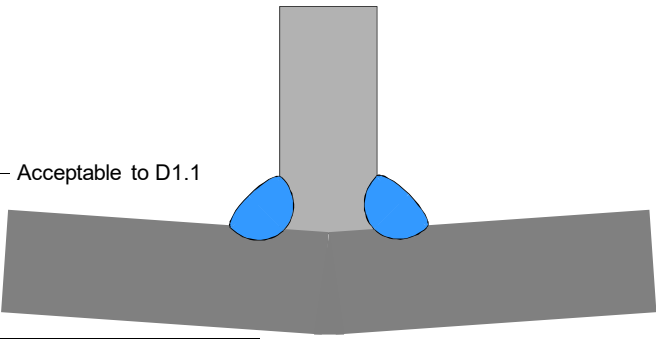
Will splice crack when insert is welded?

122

This diagram shows a cross-section of a flange CJP groove weld with a welded insert. A red circle with a diagonal line through it is placed over the top left corner. The text 'May be AESS specified practice' is at the top, and 'Will splice crack when insert is welded?' is at the bottom. The AISC logo is in the bottom left corner, and the number '122' is in the bottom right corner.

SPECIAL WELDING APPLICATIONS

Distortion



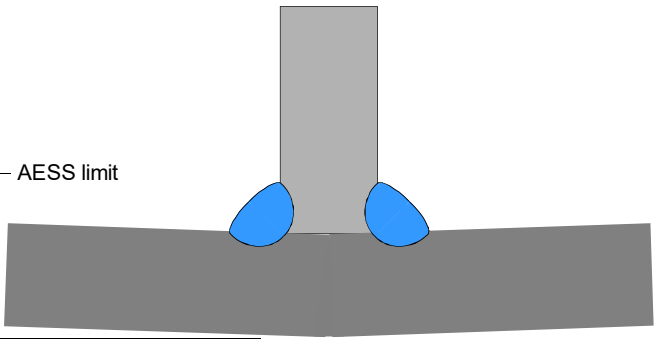
Acceptable to D1.1

123

This diagram shows a cross-section of a T-joint weld. A vertical plate is welded to a horizontal plate. The weld is shown in blue. The horizontal plate is distorted downwards. The text 'Distortion' is at the top, and 'Acceptable to D1.1' is at the bottom. The AISC logo is in the bottom left corner, and the number '123' is in the bottom right corner.

SPECIAL WELDING APPLICATIONS

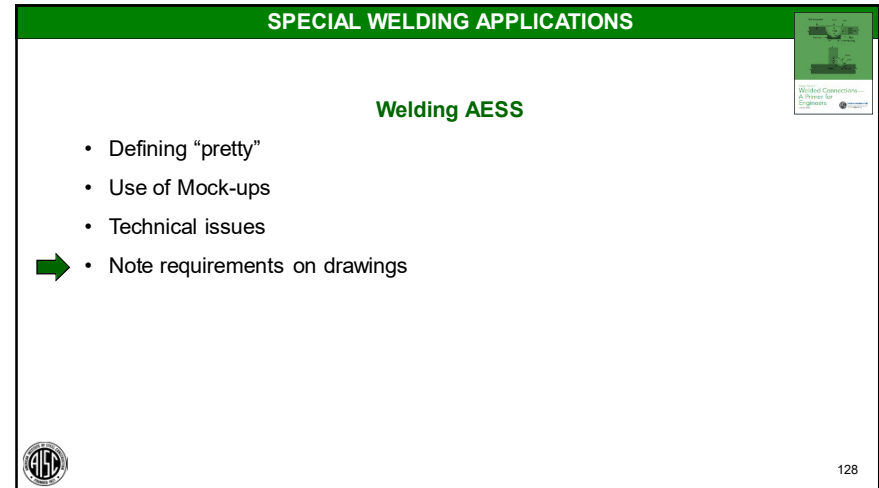
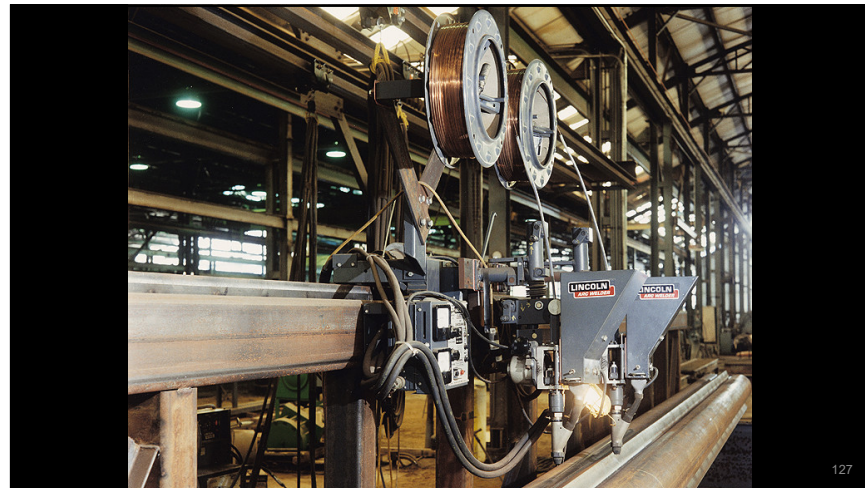
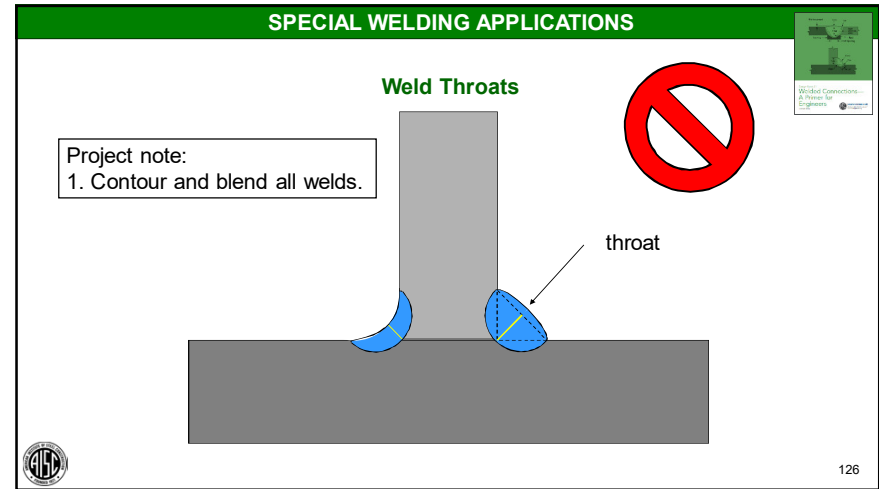
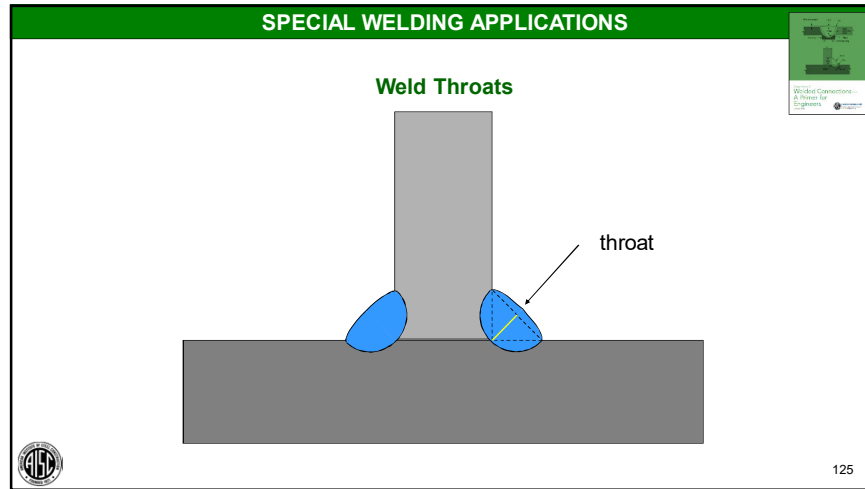
Distortion



AESS limit

124


This diagram shows a cross-section of a T-joint weld. A vertical plate is welded to a horizontal plate. The weld is shown in blue. The horizontal plate is distorted downwards. The text 'Distortion' is at the top, and 'AESS limit' is at the bottom. The AISC logo is in the bottom left corner, and the number '124' is in the bottom right corner.



SPECIAL WELDING APPLICATIONS

Welding AESS: Summary

- Defining “pretty”; use AISC 303 Section 10
- Mock-ups are often critical; view from an appropriate distance
- Define all expectations; some requirement may conflict with code requirements



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SPECIAL WELDING APPLICATIONS

Today's Webinar

- 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- 14.7 Welding AESS
- ➔ 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering




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SPECIAL WELDING APPLICATIONS

Shop versus Field Welding

- Primarily an issue of cost
- Some environmental issues (cold, wind, rain, etc.)
- Out-of-position welding may be needed
- Easier to control projects in a shop environment




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SPECIAL WELDING APPLICATIONS

Myths

- You can't get quality welds out-of-position




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AWS D1.1:2015 STRUCTURAL WELDING CODE

Table 4.12
Welding Personnel Performance Essential Variable Changes
Requiring Requalification (see 4.19)

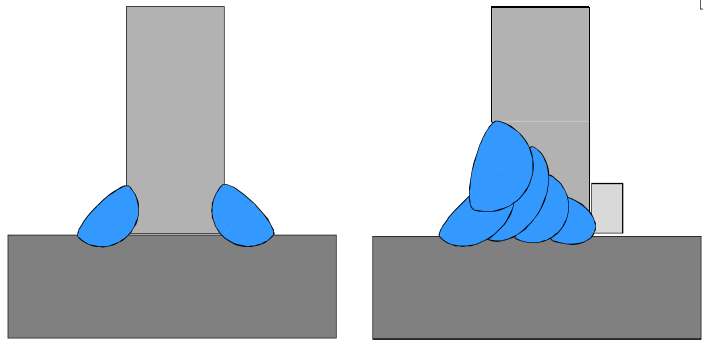

Essential Variable Changes to WPQR Requiring Requalification	Welding Personnel		
	Welders ^b	Welding Operators ^{b,c}	Tack Welders
(1) To a process not qualified (GMAW-S is considered a separate process)	X	X	X
(2) To an SMAW electrode with an F-number (see Table 4.13) higher than the WPQR electrode F-number	X		X
➔ (3) To a position not qualified	X	X	X
(4) To a diameter or thickness not qualified	X	X	
(5) To a vertical welding progression not qualified (uphill or downhill)	X		
(6) The omission of backing (if used in the WPQR test)	X	X	
(7) To multiple electrodes (if a single electrode was used in the WPQR test) but not vice versa		X ^a	



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SPECIAL WELDING APPLICATIONS

Usually Preferred

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SPECIAL WELDING APPLICATIONS

Requires overhead welding

All flat welding

Usually preferred for field work

Welded Connections – A Primer for Engineers

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SPECIAL WELDING APPLICATIONS

Myths

- You can't get quality welds out-of-position
- You can't get quality in the field
- Codes do not apply to field welding

Welded Connections – A Primer for Engineers

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AWS D1.1:2015 STRUCTURAL WELDING CODE

5. Fabrication

5.1 Scope

➔ All applicable provisions of this section shall be observed in the fabrication and erection of welded assemblies and structures produced by any process acceptable under this code (see 3.2 and 4.13).

5.2 Base Metal

5.2.1 Specified Base Metal. The contract documents shall designate the specification and classification of base metal to be used. When welding is involved in the structure, approved base metals, listed in Table 3.1 or Table 4.9, should be used wherever possible.

5.3 Welding Consumables and Electrode Requirements

5.3.1 General

5.3.1.1 Certification for Electrodes or Electrode-Flux Combinations. When requested by the Engineer, the Contractor or fabricator shall furnish certification that the electrode or electrode-flux combination conforms to the requirements of the classification.

5.3.1.2 Suitability of Classification. The classification and size of electrode, arc length, voltage, and amperage shall be suited to the thickness of the material, type of groove, welding positions, and other circumstances attending the work. Welding current shall be within the range recommended by the electrode manufacturer.

Welded Connections – A Primer for Engineers

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AWS D1.1:2015 STRUCTURAL WELDING CODE

5.1 Scope

All applicable provisions of this section shall be observed in the **fabrication and erection** of welded assemblies and structures produced by any process acceptable under this code (see 3.2 and 4.14).



Welded Connections – A Primer for Engineers

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SPECIAL WELDING APPLICATIONS

Myths

- You can't get quality welds out-of-position
- You can't get quality in the field
- Codes do not apply to field welding
- Field welders are not "certified" (qualified)
- WPSs are not used in the field
- There is no audit program for field contractors



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American Institute of Steel Construction, Inc.
is proud to recognize
ABC Erectors, Inc.
Any City, USA
for successfully meeting the quality certification requirements for
Advanced Certified Steel Erector



Roger E. Farsh



Robbie Mancolar

Certification valid through May 2026





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SPECIAL WELDING APPLICATIONS

Myths

- You can't get quality welds out-of-position
- You can't get quality in the field
- Codes do not apply to field welding
- Field welders are not "certified" (qualified)
- WPSs are not used in the field
- There is no audit program for field contractors
- There are no contractor-supplied inspectors for field work





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AISC 360-16 Specification for Structural Steel Buildings

CHAPTER N
QUALITY CONTROL AND QUALITY ASSURANCE

N1. GENERAL PROVISIONS

Quality control (QC) as specified in this chapter shall be provided by the **fabricator and erector**. Quality assurance (QA) as specified in this chapter shall be provided by others when required by the authority having jurisdiction (AHJ), applicable building code, purchaser, owner, or engineer of record (EOR). Nondestructive testing (NDT) shall be performed by the agency or firm responsible for quality assurance, except as permitted in accordance with Section N6.



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AISC 360-16 Specification for Structural Steel Buildings

CHAPTER N

QUALITY CONTROL AND QUALITY ASSURANCE

N2. FABRICATOR AND ERECTOR QUALITY CONTROL PROGRAM

The **fabricator and erector** shall establish, maintain and implement QC procedures to ensure that their work is performed in accordance with this Specification and the construction documents.



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SPECIAL WELDING APPLICATIONS

Shop vs Field Welding: Summary

- There are some legitimate differences/concerns
- Shop operations generally preferred due to lower costs
- Mature codes control field welding
- Field contractor certification programs exist
- Inspection requirements have been made clearer in Chapter N



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SPECIAL WELDING APPLICATIONS

Today's Webinar

- 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- ➔ 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering





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SPECIAL WELDING APPLICATIONS

14.9 Welding on Existing Structures

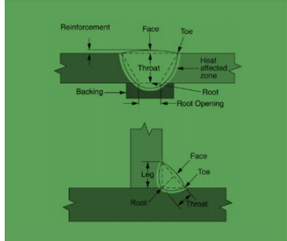
- ➔ • Historic steels
- Welding under load
- Fire
- Cold working / strain aging




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AISC Design Guide 21
Chapter 4 Metallurgical Issues

- Historic (obsolete) steels
 - ASTM A9
 - ASTM A7
 - ASTM A373
 - ASTM A242
- Check weldability of steel (especially if riveted)



Design Guide 21
**Welded Connections—
A Primer for
Engineers**
Smarter. Stronger. Steel.
American Institute of Steel Construction, Inc.




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SPECIAL WELDING APPLICATIONS


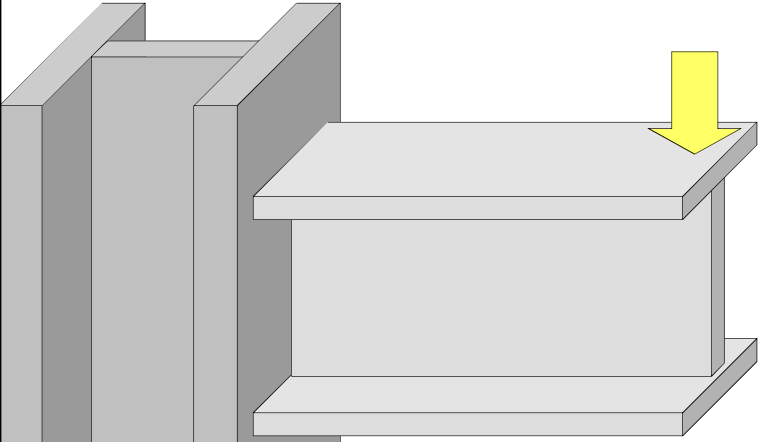
14.9 Welding on Existing Structures

- Historic steels
- Welding under load
- Fire
- Cold working / strain aging




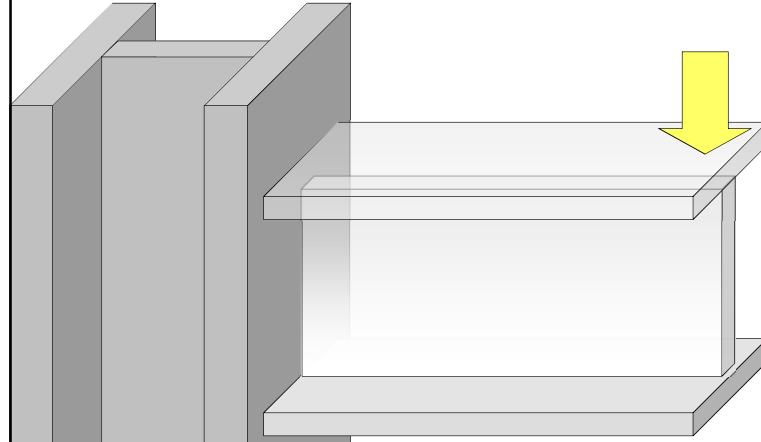
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SPECIAL WELDING APPLICATIONS

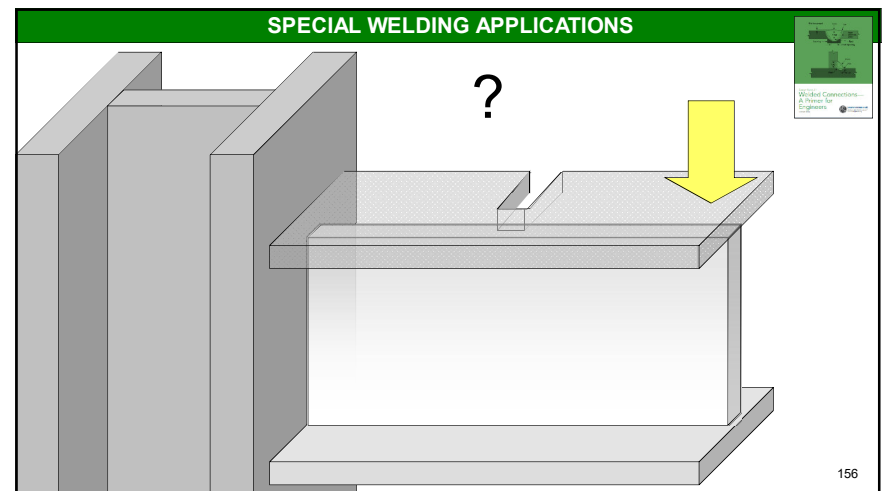
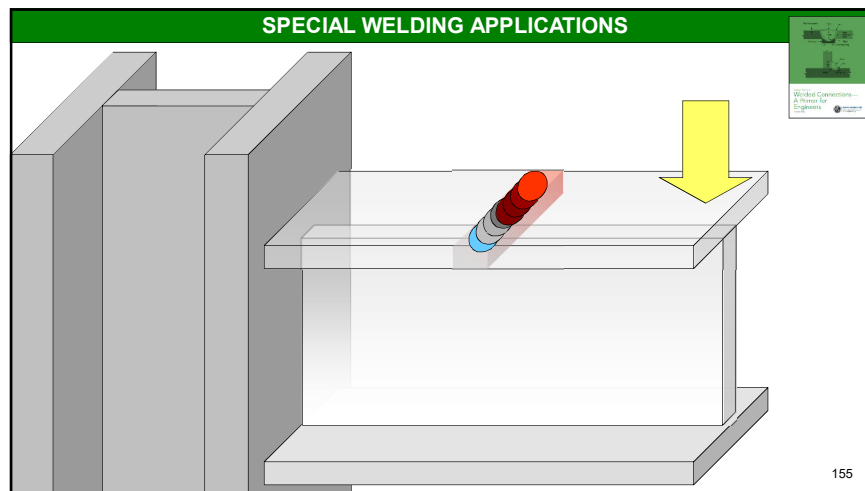
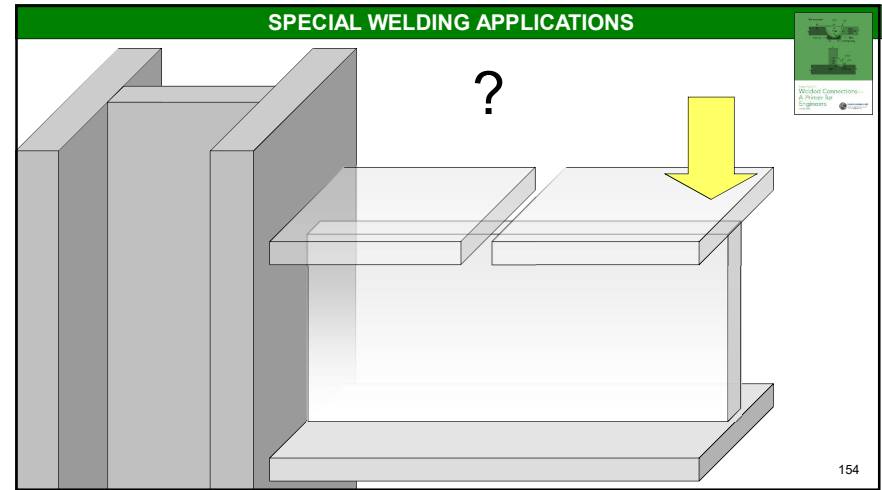
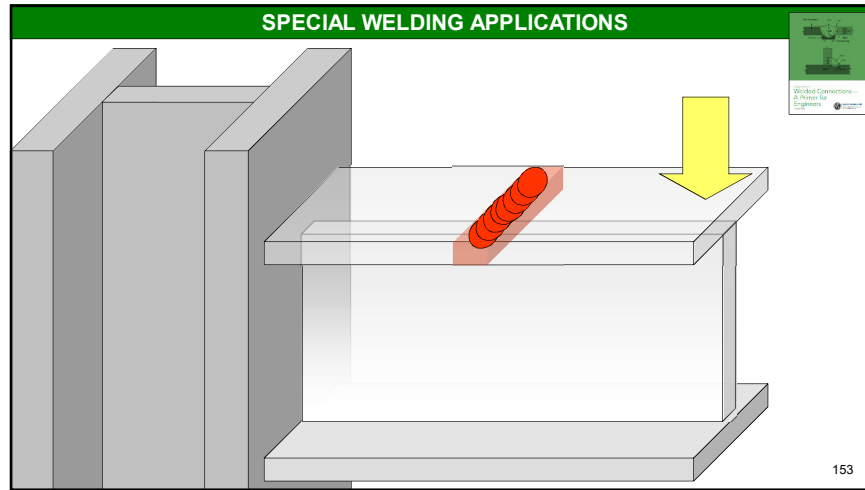


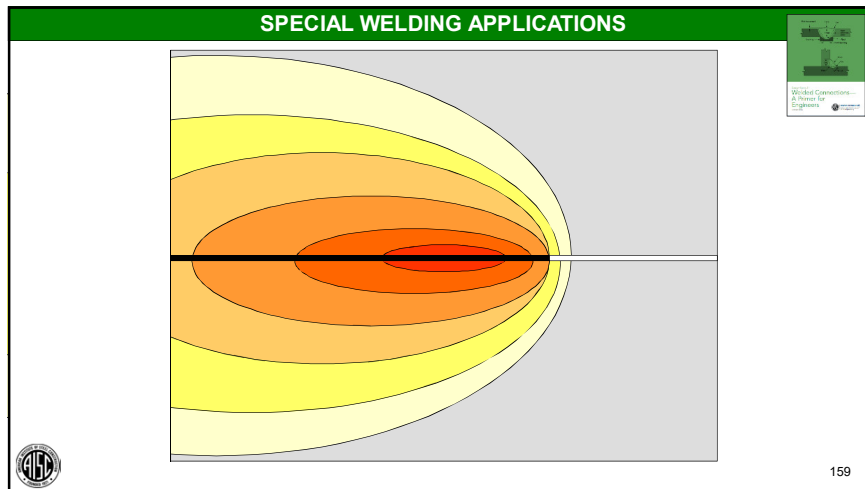
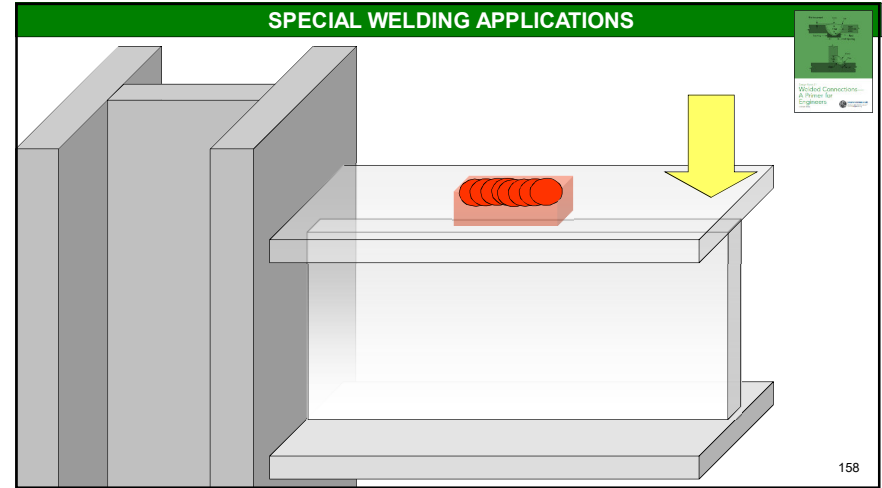
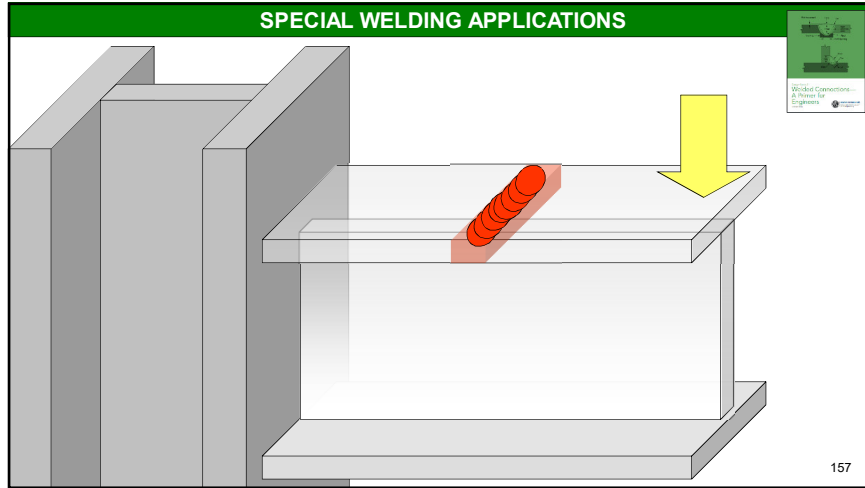
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SPECIAL WELDING APPLICATIONS



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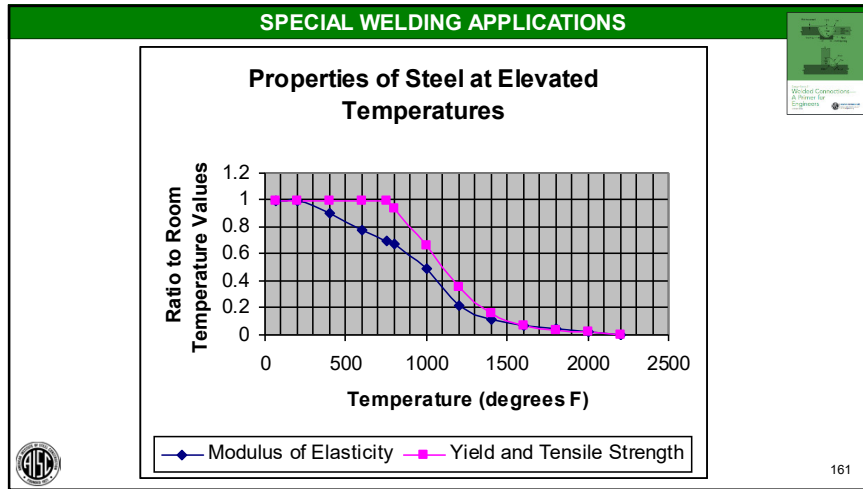
AISC 360-16 Specification for Structural Steel Buildings

Table A-4.2.1
Properties of Steel at Elevated Temperatures

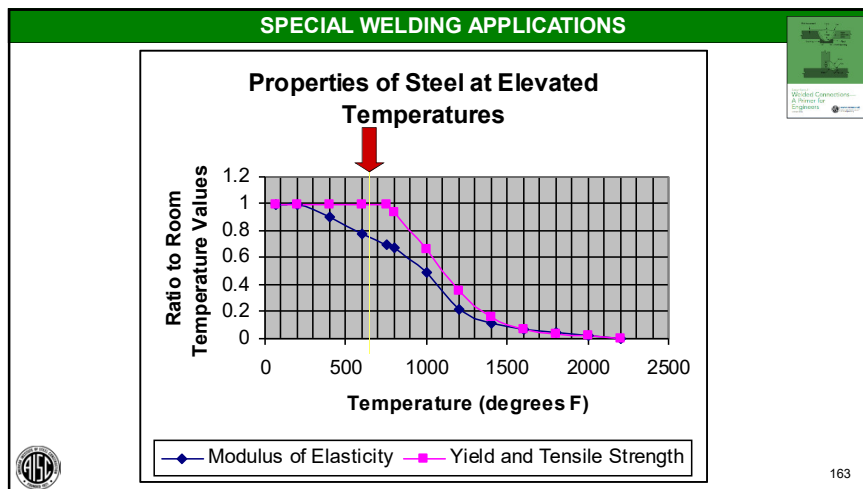
Steel Temperature (°F)[°C]	$k_E = E_m/E$	$k_y = F_{ym}/F_y$	$k_u = F_{um}/F_y$
68 [20]	*	*	*
200 [93]	1.00	*	*
400 [204]	0.90	*	*
600 [316]	0.78	*	*
750 [399]	0.70	1.00	1.00
800 [427]	0.67	0.94	0.94
1000 [538]	0.49	0.66	0.66
1200 [649]	0.22	0.35	0.35
1400 [760]	0.11	0.16	0.16
1600 [871]	0.07	0.07	0.07
1800 [982]	0.05	0.04	0.04
2000 [1093]	0.02	0.02	0.02
2200 [1204]	0.00	0.00	0.00

*Use ambient properties.

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- SPECIAL WELDING APPLICATIONS**
- Welding on members under load**
- The amount of material at temperatures >650 °F is negligible (Blodgett)
 - Only a very small percentage of the cross section experiences reduced properties (Tide)
 - The impact of the weld orientation (longitudinal versus transverse) is typically inconsequential (Ricker)
 - Each situation should be checked
- 162




- SPECIAL WELDING APPLICATIONS**
- 14.9 Welding on Existing Structures**
- Historic steels
 - Welding under load
 - ➔ • Fire
 - Cold working / strain aging
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SPECIAL WELDING APPLICATIONS

Welding on Existing Structures

- General precaution: Fire!
 - From combustibles
 - From unintended work circuits
 - Thermal cutting is typically >> problem than welding




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SPECIAL WELDING APPLICATIONS

14.9 Welding on Existing Structures

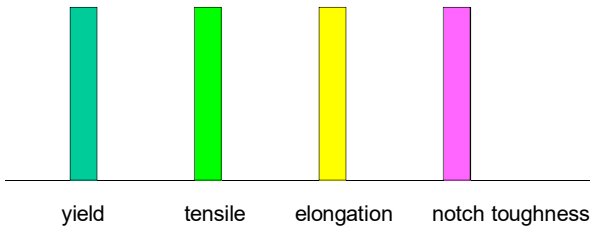
- Historic steels
- Welding under load
- Fire
- • Cold working / strain aging




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SPECIAL WELDING APPLICATIONS

As received hot rolled steel



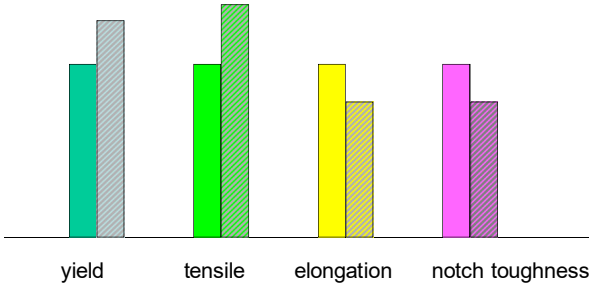
Property	Value (Relative)
yield	Medium
tensile	Medium
elongation	Medium
notch toughness	Medium




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SPECIAL WELDING APPLICATIONS

Effects of cold working



Property	Original State	After Cold Working
yield	Medium	High
tensile	Medium	High
elongation	Medium	Low
notch toughness	Medium	Low




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SPECIAL WELDING APPLICATIONS

Strain Aging

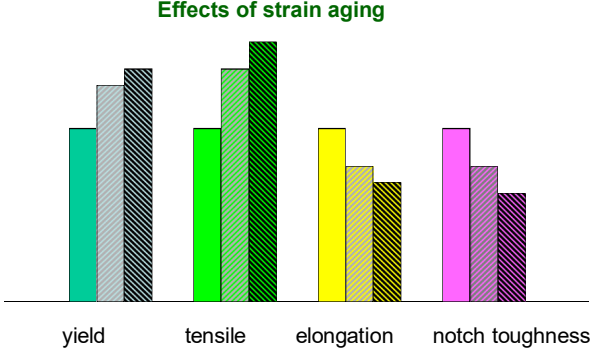
- Occurs when steel is heated to 400-700 °F
- Yield, tensile increase
- Ductility, notch toughness decrease
- Aggravated by presence of “free” nitrogen
- Stress relief helps, but...
 - Typically impractical
 - Depending on alloy, may experience cracking (Cr, Mo, V, B)




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SPECIAL WELDING APPLICATIONS

Effects of strain aging



Property	Effect
yield	Increases
tensile	Increases
elongation	Decreases
notch toughness	Decreases




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SPECIAL WELDING APPLICATIONS

Welding on plastically deformed members

- Reduced notch toughness
 - Make sure material is crack and notch free
- Reduced ductility
 - Minimize practices that increase ductility demand



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AWS D1.1:2015 STRUCTURAL WELDING CODE

8. Strengthening and Repair of Existing Structures

8.1 General

Strengthening or repairing an existing structure shall consist of modifications to meet design requirements specified by the Engineer. The Engineer shall prepare a comprehensive plan for the work. Such plans shall include, but are not limited to, design, workmanship, inspection, and documentation. Except as modified in this section, all provisions of this code shall apply equally to the strengthening and repairing of existing structures, including heat straightening of distorted members.

8.2 Base Metal


8.2.1 Investigation. Before preparing drawings and specifications for strengthening or repairing existing structures, the types of base metal used in the original

8.3.2 Stress Analysis. An analysis of stresses in the area affected by the strengthening or repair shall be made. Stress levels shall be established for all in-situ dead and live load cases. Consideration shall be made for accumulated damage that members may have sustained in past service.

8.3.3 Fatigue History. Members subject to cyclic loading shall be designed according to the requirements for fatigue stresses. The previous loading history shall be considered in the design. When the loading history is not available, it shall be estimated.

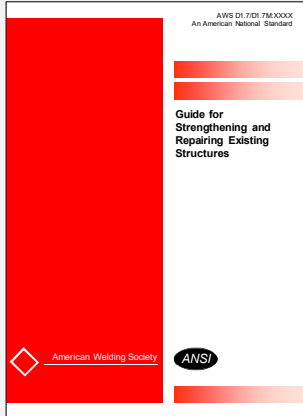
8.3.4 Restoration or Replacement. Determination shall be made whether the repairs should consist of restoring corroded or otherwise damaged parts or of replacing entire members.

8.3.5 Loading During Operations. The Engineer shall



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AWS D1.7
Guide for Strengthening and Repairing Existing Structures



AWS D1.7:2017
An American National Standard

Guide for Strengthening and Repairing Existing Structures

American Welding Society ANSI

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SPECIAL WELDING APPLICATIONS

Welding on Existing Structures: Summary

- May involve historic steel: see DG 21 Chapter 4
- Welding on members under load: Caution
- Fire potential: sparks, resistance heating: Caution
- Welding on cold worked steel: local reduced ductility

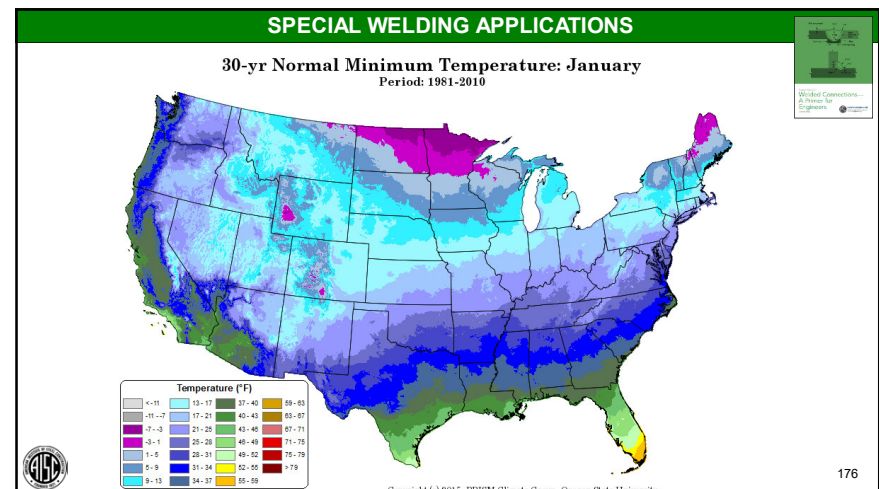
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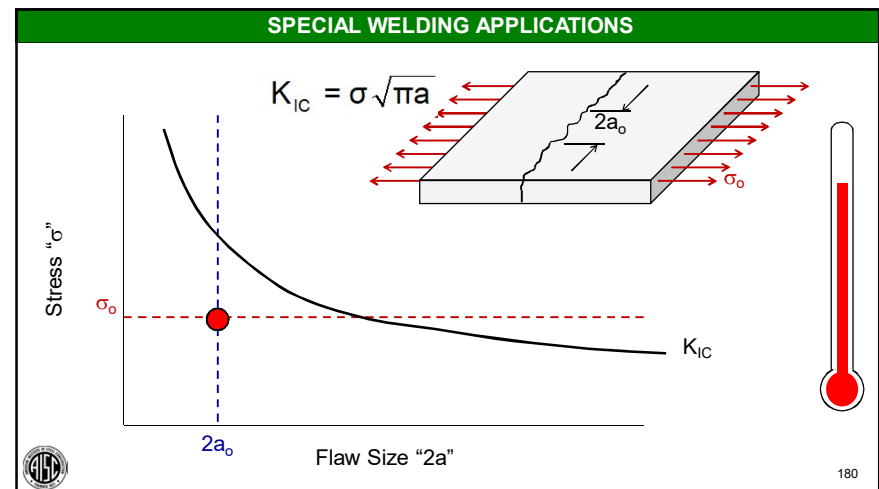
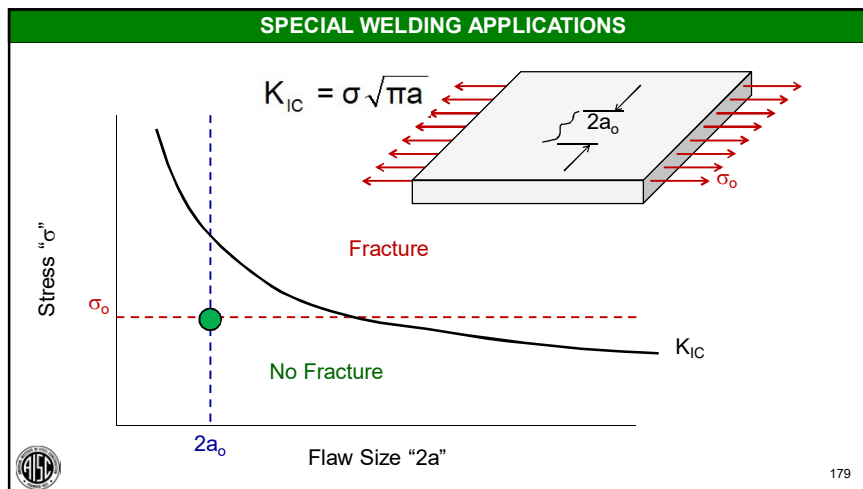
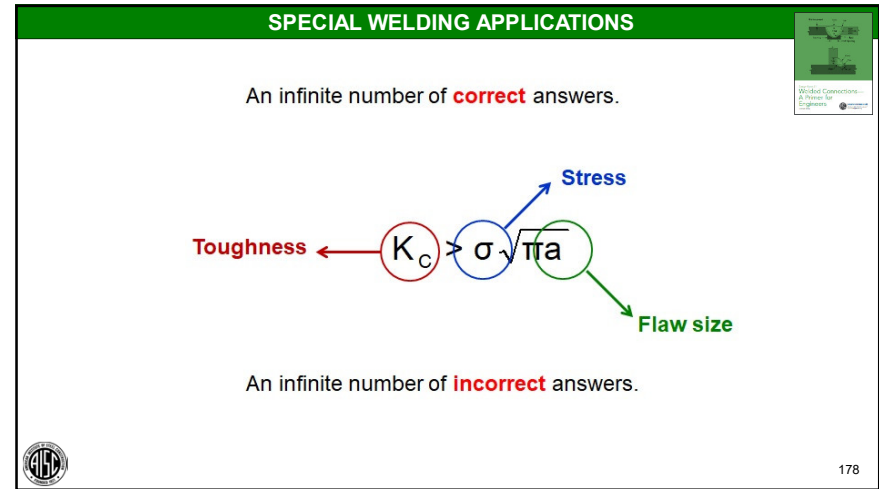
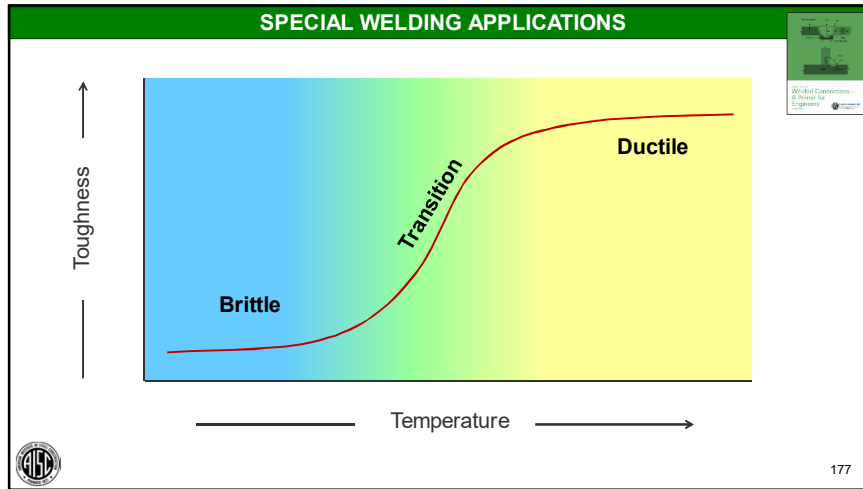
SPECIAL WELDING APPLICATIONS

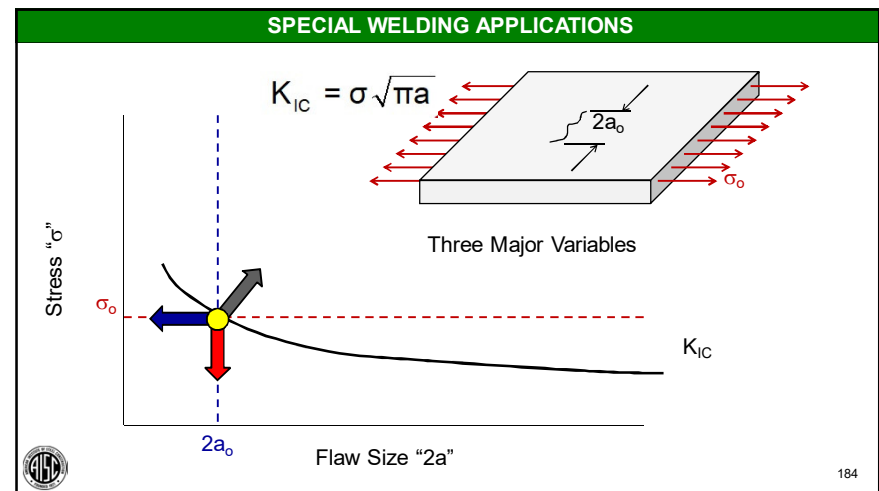
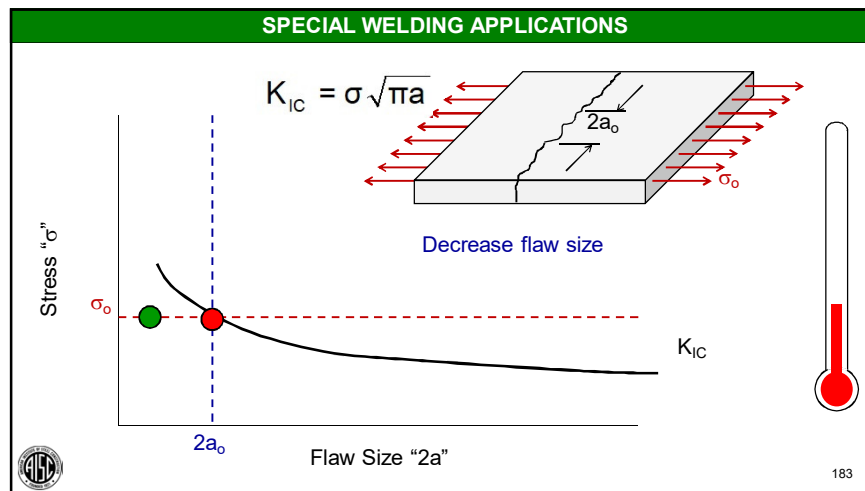
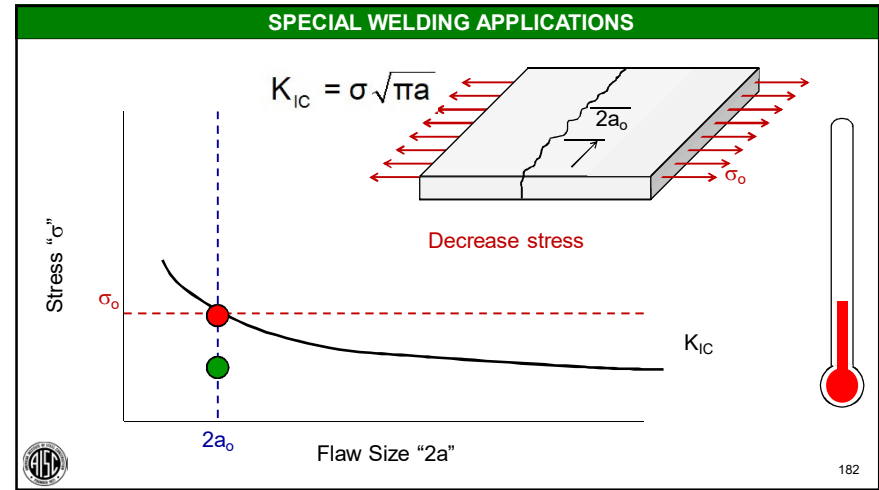
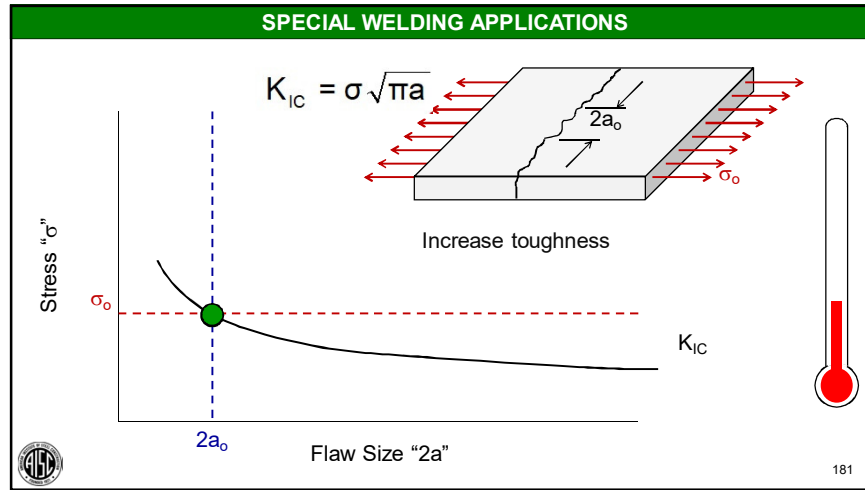
Today's Webinar

- 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- ➔ 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering

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AISC 360-16 Specification for Structural Steel Buildings

Commentary A3. MATERIAL

1a. ASTM Designations

“For especially demanding service conditions such as structures **exposed to low temperatures**, particularly those with impact loading, the specification of **steels with superior notch toughness may be warranted.**”

$$K_c > \sigma\sqrt{\pi a}$$

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AISC 360-16 Specification for Structural Steel Buildings

Commentary A3. MATERIAL

1a. ASTM Designations

“**Good workmanship** and **good design details** incorporating joint geometry that **avoids severe stress concentrations** are generally the **most effective means of providing fracture-resistant construction.**”

$$K_c > \sigma\sqrt{\pi a}$$

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SPECIAL WELDING APPLICATIONS

Cold Temperature Applications: Summary

Material

Details

Workmanship

Inspection

For “good design details” and joint geometries that “avoid severe stress concentrations”, consider Appendix 3 Design for Fatigue.

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SPECIAL WELDING APPLICATIONS

Today’s Webinar


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SPECIAL WELDING APPLICATIONS

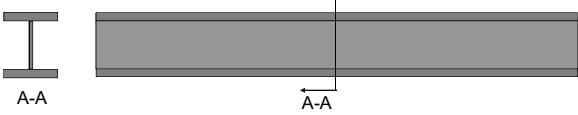
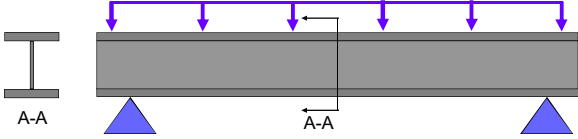

Heat Shrinking

- Used to curve straight steel
- Used to straighten bent steel
- Used to correct for distortion
- Also known as “heat straightening”



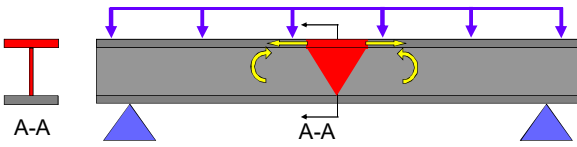
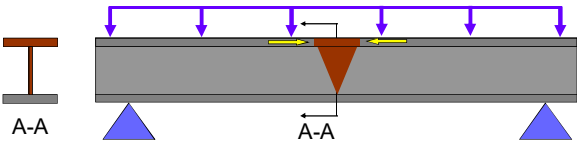

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SPECIAL WELDING APPLICATIONS

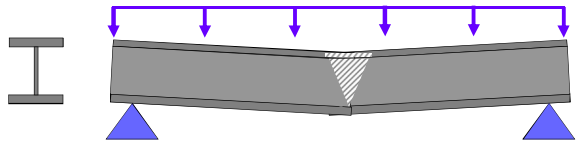
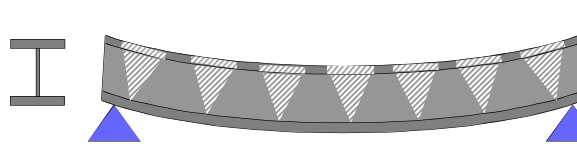

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SPECIAL WELDING APPLICATIONS

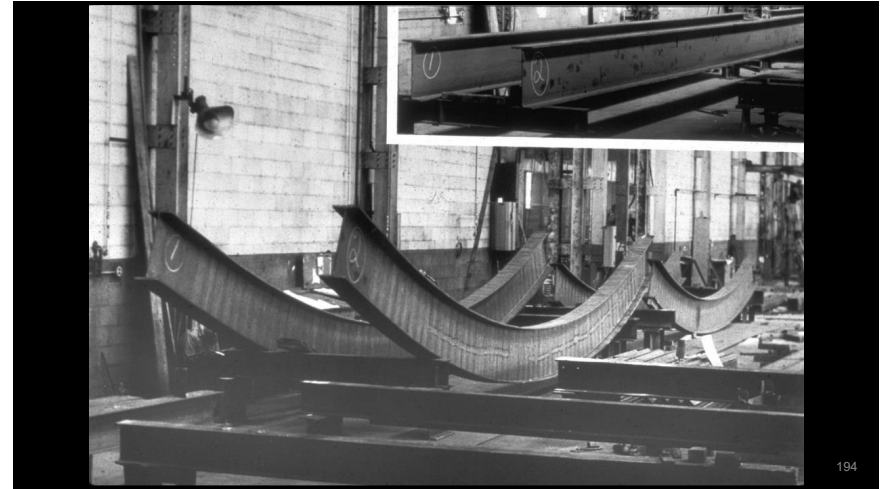
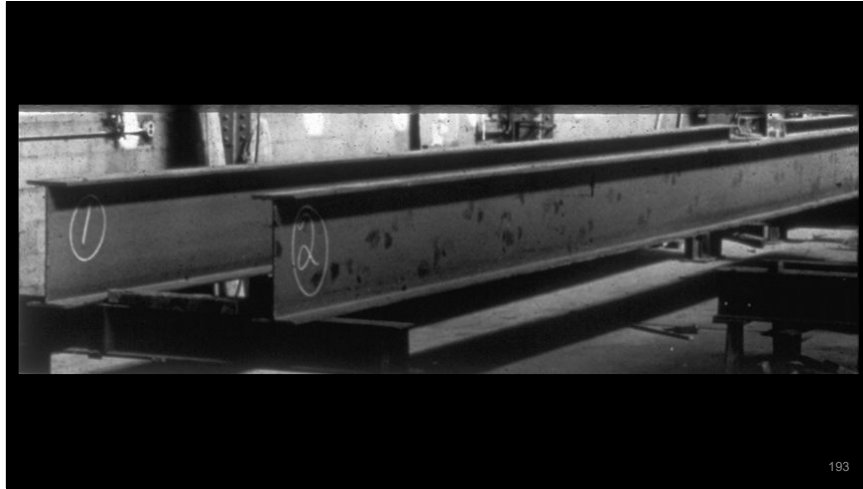




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SPECIAL WELDING APPLICATIONS



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SPECIAL WELDING APPLICATIONS

Heat Shrinking

- 1200 °F temperature limit for hot rolled steels
- 1100 °F temperature limit for quenched and tempered steels
- Pre-stress of up to 50% of room temperature yield (Avent)




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AWS D1.1:2015 STRUCTURAL WELDING CODE

5.25.2 Localized Heat Repair Temperature Limitations.

Members distorted by welding shall be straightened by mechanical means or by application of a limited amount of localized heat. The temperature of heated areas as measured by approved methods shall not exceed 1100°F [600°C] for quenched and tempered steel nor 1200°F [650°C] for other steels. The part to be heated for straightening shall be substantially free of stress and from external forces, except those stresses resulting from the mechanical straightening method used in conjunction with the application of heat.




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SPECIAL WELDING APPLICATIONS

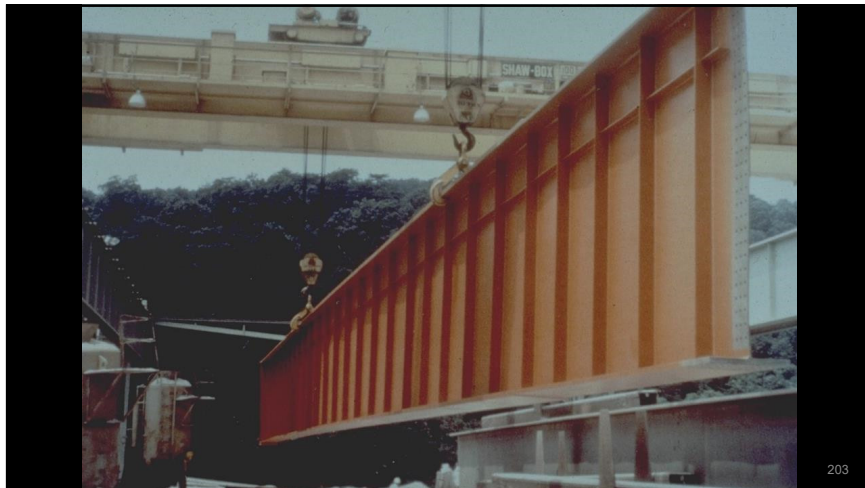
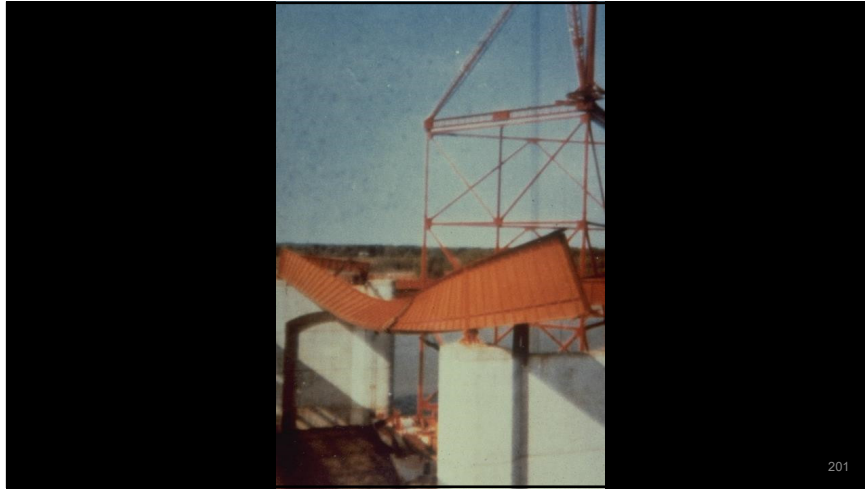
Heat Shrinking

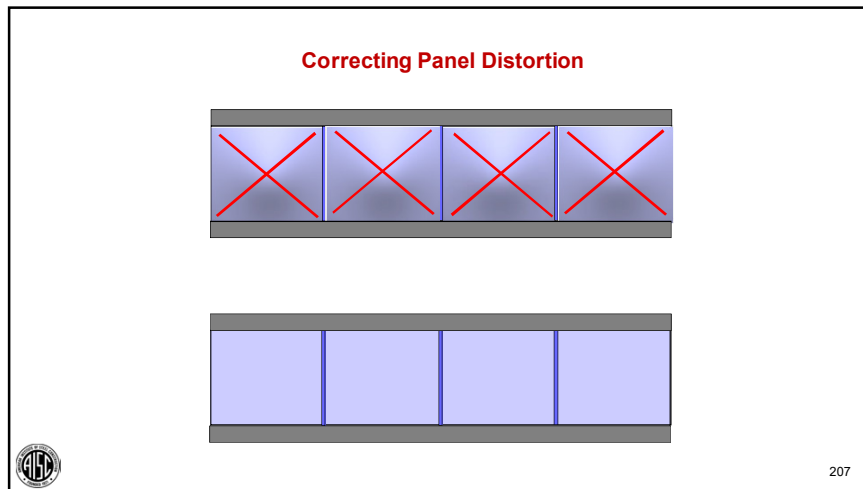
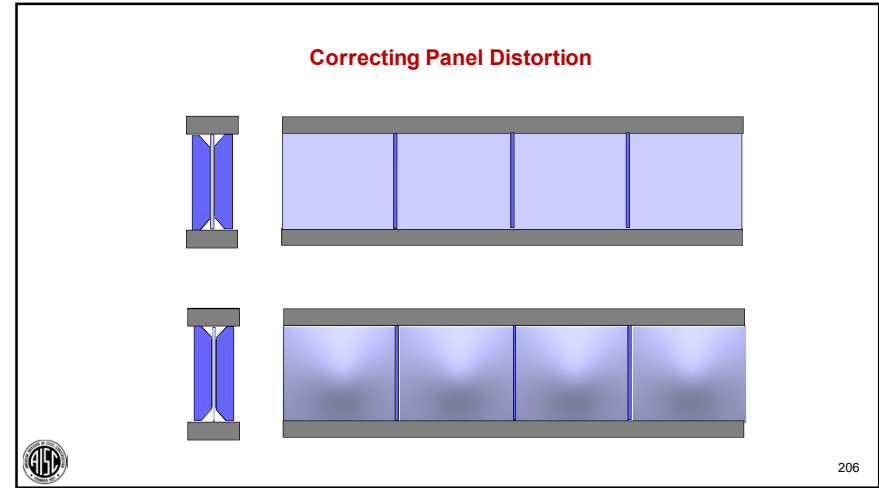
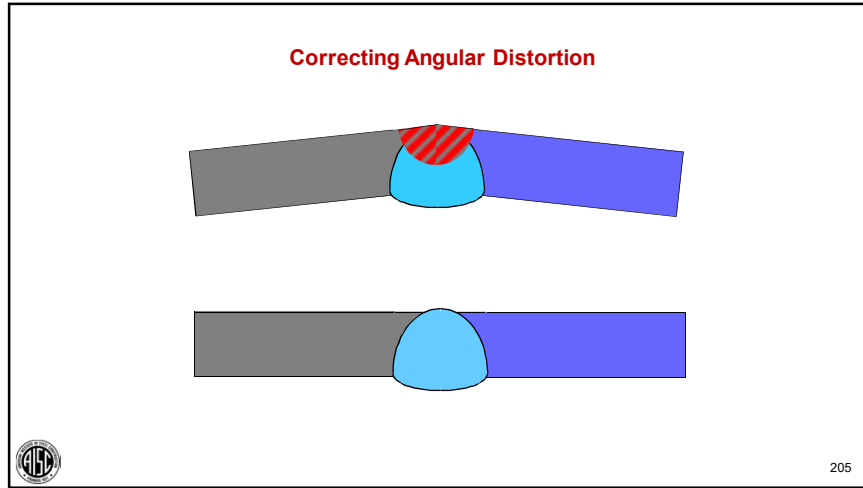
- For new steel being curved
 - No change in modulus of elasticity (E)
 - Slight increase in yield and tensile strength
 - 10-25% increase in ductility
- For bent steel being straightened
 - Yield strength increases 10%
 - Tensile strength increases 4-6%



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SPECIAL WELDING APPLICATIONS

Heat Shrinking: Summary

- Used for shaping and straightening steel
- Code limits on temperatures
- Material is not damaged if properly done

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SPECIAL WELDING APPLICATIONS

Today's Webinar

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- ➔ 14.16 Buttering



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SPECIAL WELDING APPLICATIONS

BUTTERING





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AWS STANDARD WELDING TERMS & DEFINITIONS (A3.0:2010)

buttering.

A surface variation depositing surfacing metal on one or more surfaces to provide metallurgically compatible weld metal for the subsequent completing of the weld.





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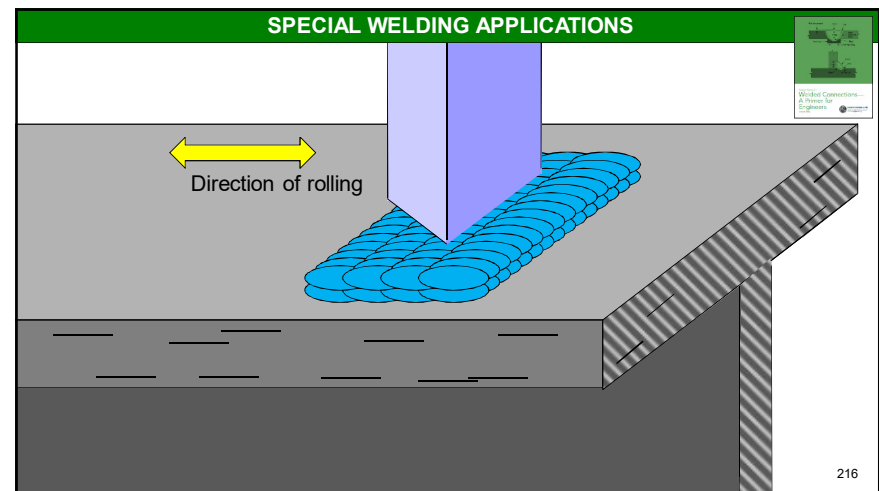
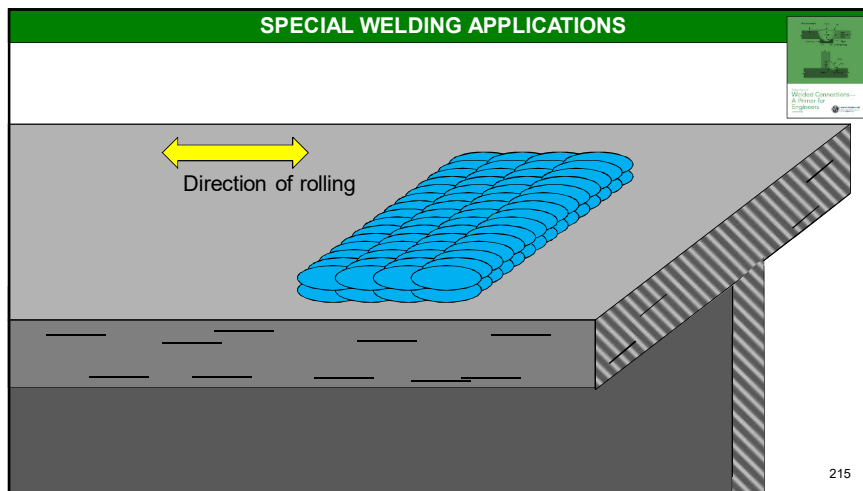
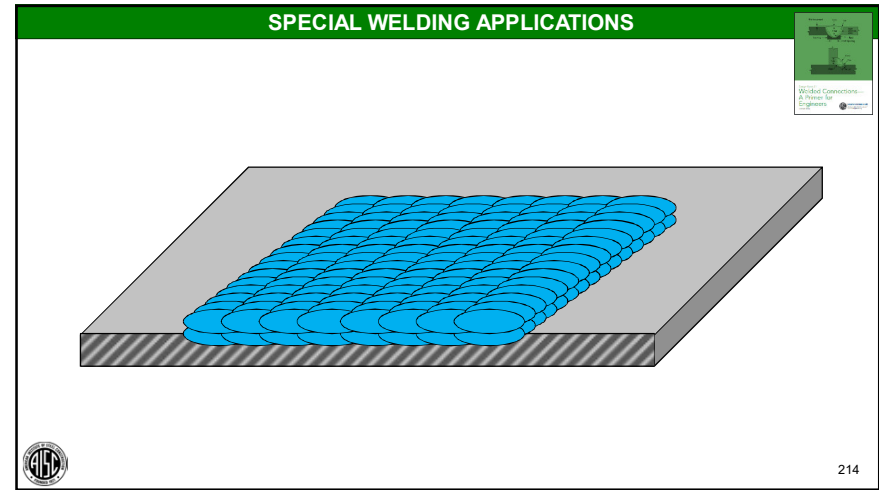
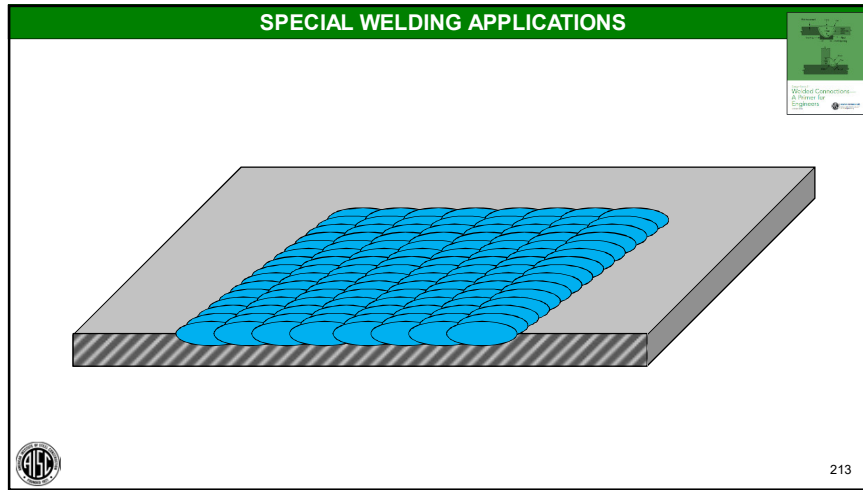
SPECIAL WELDING APPLICATIONS

Buttering is used to:

- Mitigate lamellar tearing tendencies
- Build up the faces of groove weld joint cavities where root openings are excessive
- Restore material from steel that was damaged by corrosion
- Enable welding on "hard to weld" steels

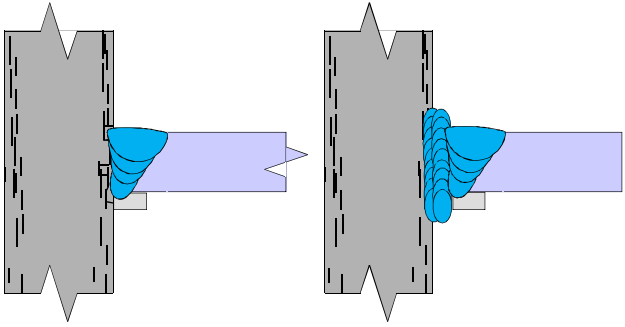


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SPECIAL WELDING APPLICATIONS

Buttering Solution



Sensitive Detail Buttered Detail

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SPECIAL WELDING APPLICATIONS

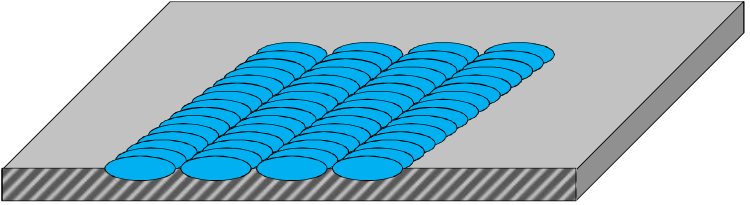
Buttering Guidelines

- Deposit butter layers with the same care and controls used for joining welds (WPSs, qualified welders, in process visual inspection, etc.)
- Overlap bead by approximately 50%

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SPECIAL WELDING APPLICATIONS

Non-preferred: no overlap

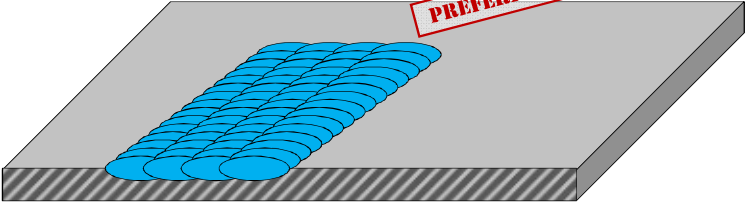


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SPECIAL WELDING APPLICATIONS

Approximately 50% overlap

PREFERRED





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SPECIAL WELDING APPLICATIONS

Buttering Guidelines

- Deposit butter layers with the same care and controls used for joining welds (WPSs, Qualified welders, in process visual inspection, etc.)
- Overlap bead by approximately 50%.
- If NDT of the weld to be made to the butter layer is required, the butter layer should also receive NDT.





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SPECIAL WELDING APPLICATIONS

Buttering: Summary

- Build up out-of-tolerance joints
- One method to mitigate lamellar tearing
- Buttering procedures should be consistent with the applicable code, including final quality requirements



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SPECIAL WELDING APPLICATIONS

Today's Webinar

- 14.4 Welding on Heavy Shapes
- 14.6 Welding HSS
- 14.7 Welding AESS
- 14.8 Shop Versus Field Welding
- 14.9 Welding on Existing Structures
- 14.12 Cold Temperature Applications
- 14.15 Heat Shrinking
- 14.16 Buttering



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SPECIAL WELDING APPLICATIONS



224

Thank you!

AISC | Questions?



Individual Session Registrants

PDH Certificates

- You will receive an email on how to report attendance from: registration@aisc.org.
- Be on the lookout: Check your spam filter! Check your junk folder!
- Completely fill out online form. Don't forget to check the boxes next to each attendee's name!



Individual Session Registrants

PDH Certificates

- Reporting site (URL will be provided in the forthcoming email).
- Username: Same as AISC website username.
- Password: Same as AISC website password.



8-Session Registrants

PDH Certificates

One certificate will be issued at the conclusion of all 8 sessions.



8-Session Registrants

Access to the quiz

Information for accessing the quiz will be emailed to you by Thursday. It will contain a link to access the quiz. EMAIL COMES FROM NIGHTSCHOOL@AISC.ORG.

Quiz and attendance records

Posted Thursday mornings. www.aisc.org/nightschool -- Click on Current Course Details.

Reasons for quiz

- EEU – You must take all quizzes and the final exam to receive EEU.
- PDHs – If you watch a recorded session, you must pass quiz for PDHs.
- REINFORCEMENT – Reinforce what you learn tonight. Get more out of the course.

Note: If you attend the live presentation, you do not have to take the quizzes to receive PDHs



8-Session Registrants

Access to the recording

Information for accessing the recording will be emailed to you by Thursday. The recording will be available for four weeks. (For 8-session registrants only.) EMAIL COMES FROM NIGHTSCHOOL@AISC.ORG.

PDHs via recording

If you watch a recorded session, you must take *and pass* the quiz for PDHs.



8-Session Registrants

Night School Resources

Find all your handouts, quizzes and quiz scores, recording access, and attendance information all in one place!



8-Session Registrants

Night School Resources

Go to www.aisc.org and sign in.



Login

If you're an existing customer, please enter your username and password.

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Enter your username

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Enter your password

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[REGISTER NOW](#)



8-Session Registrants

Night School Resources

Go to www.aisc.org and sign in.

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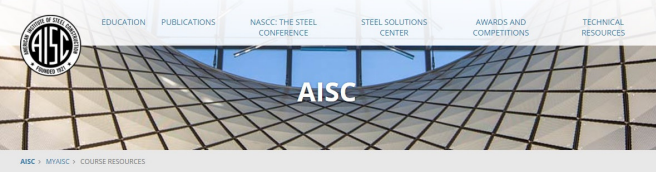
MY COURSE RESOURCES
View online resources for Night School and Live Webinar package registrations.
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Smarter. Stronger. Steel.


8-Session Registrants

Night School Resources



Course Resources


Event	Start Date
NS 13 8-Session Package-Night School 13 - Design of Industrial Buildings	1/30/2017 7:00:00 PM
NS 14 8-Session Package-Night School 14 - Fundamentals of Stability	6/9/2017 7:00:00 PM



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8-Session Registrants


Night School Resources



Night School 13: Design of Industrial Buildings

8-SESSION PACKAGE RESOURCES

Event	Date	Handouts	Videos	Quiz	Attendance
NS13 - Design Criteria	1/30/2017 7:00:00 PM	Download	Video	Pass Score 80	Pending
NS13 - Economic Considerations	2/4/2017 7:00:00 PM	Download	Available 02/08/2017 5pm EST	Available 02/08/2017 5pm EST	Pending
NS13 - Lateral Load Systems and Details	2/15/2017 7:00:00 PM	Download	Available 02/15/2017 5pm EST	Available 02/15/2017 5pm EST	Pending
NS13 - Preliminary Design Procedures	2/27/2017 7:00:00 PM	Download	Available 03/05/2017 5pm EST	Available 03/05/2017 5pm EST	Pending
NS13 - Crane Girder Design and Frame Analysis	3/6/2017 7:00:00 PM	Download	Available 03/06/2017 5pm EST	Available 03/06/2017 5pm EST	Pending
NS13 - Frame Member and Connection Design	3/13/2017 7:00:00 PM	Download	Available 03/15/2017 5pm EST	Available 03/15/2017 5pm EST	Pending
NS13 - Transfer Crane Girder & Longitudinal Bracing Design	3/27/2017 7:00:00 PM	Download	Available 03/28/2017 5pm EST	Available 03/29/2017 5pm EST	Pending




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8-Session Registrants

Night School Resources

- Weekly “quiz and recording” email.
- Weekly updates of the master quiz and attendance record, found at www.aisc.org/nightschool21. Scroll down to Quiz and Attendance records.
 - Updated on Thursday mornings.

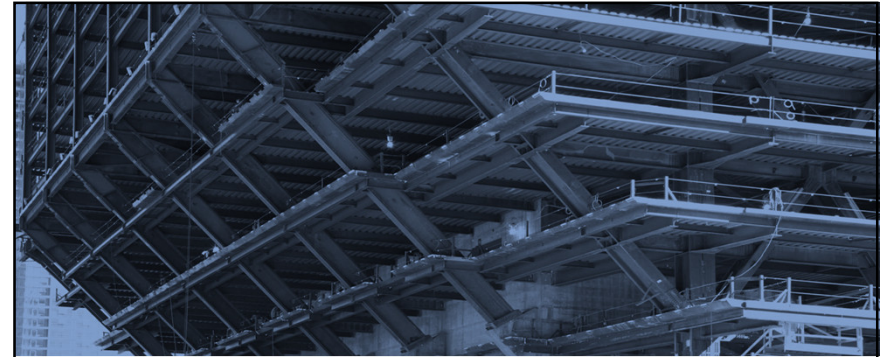


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8-Session Registrants

Night School Resources

- Webinar connection information
 - Reminder email sent out Tuesday mornings
- Links to handouts also found here



AISC | Thank you

